

High Country Water Quality Initiative



High Country Council of Governments September 2012

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HIGH COUNTRY REGIONAL WATER QUALITY INITIATIVE

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HIGH COUNTRY REGIONAL WATER QUALITY INITIATIVE

Narrative

I. Introduction

The High Country Regional Water Quality Initiative project consists of a series of tasks designed to advance water quality protection in the High Country region of North Carolina. The High Country region includes Alleghany, Ashe, Avery, Mitchell, Watauga, Wilkes, and Yancey Counties. The region is rural, mountainous, and heavily-dissected by streams. The New, Watauga, French Broad, Catawba, and Yadkin Rivers all have headwaters in the region. There are 19 Towns in the region, none of which are subject to EPA Phase I or Phase II Stormwater Program regulations.

The High Country Regional Water Quality Initiative project included the following work elements:

- Review of existing local government ordinances related to stormwater management
- Mapping of property owned by local governments, to aid in stormwater BMP site selection
- Consultation with local government staff to identify feasible stormwater BMP sites
- Consultation with local Soil and Water Conservation District staff to identify feasible stormwater BMP sites
- Development of GIS datasets depicting town and county-owned properties, catchment areas, and BMP sites
- Development of site-specific stormwater BMP site plans
- Presentation to local governments of model ordinance addressing stormwater management
- Creation of GIS datasets depicting erodible soils, steep slopes, and impervious surfaces in the region
- Edit of hydrology data to accurately reflect flowline characteristics within the BMP drainage area based on field observations.

II. Stormwater Best Management Practices

A total of 26 stormwater BMP plans were developed. The BMP plans are site specific, and are recommended based on available property, feasibility to construct, potential to reduce pollutants from entering streams, and demonstration value.

The project initially intended to develop plans for agricultural BMPs and streambank restoration projects. Following discussions with Soil and Water Conservation District

(SWCD) and Natural Resources Conservation Service (NRCS) staff and Boards throughout the region, it was determined that the agricultural community is already being served with technical and financial assistance through existing, proven SWCD and NRCS programs. SWCD and NRCS staff assists the farming community in developing BMPs funded through the NC Agricultural Cost Share Program and the Community Conservation Assistance Program.

High Country Council of Governments staff assessed approximately 100 potential BMP sites throughout the region. Feasible sites were not identified in all jurisdictions, and multiple sites were identified in some jurisdictions. Stormwater BMP plans were developed for the following sites:

1. Alleghany County – Alleghany County EMS Station
2. Town of Sparta – Town Maintenance Facility
3. Ashe County – Ashe County Courthouse
4. Town of Jefferson – Ashe Memorial Hospital
5. Town of Jefferson – Jefferson Fire Station/Gates Corporation
6. Town of Lansing – Bridge at Old Field Branch
7. Town of West Jefferson – Bowie Seagraves Park
8. Avery County – Banner Cabinets/Avery County Ballfield
9. Town of Banner Elk – Tate Evans Park
10. Town of Crossnore – Town Center/Crossnore School
11. Town of Newland – Old Public Road/North Toe River
12. Village of Sugar Mountain – Craggy Pointe
13. Mitchell County – Mitchell County Senior Center
14. Town of Bakersville – Creekwalk Trailhead
15. Town of Bakersville – Hemlock Drive Parking Lot
16. Town of Spruce Pine – Brad Ragan Park
17. Watauga County – Cooperative Extention/EMS Parking Lot
18. Town of Blowing Rock – Maple Street Parking Lot
19. Town of Boone – Appalachian State University, Duncan Hall
20. Wilkes County – Wilkes County Senior Center
21. Town of Ronda – Memorial Park
22. Town of Wilkesboro – Tyson Plant (Main/Cherry Streets)
23. Town of Wilkesboro – Wilkesboro United Methodist Church
24. Yancey County – Yancey County Health Department
25. Yancey County – Ray Cort Park
26. Town of Burnsville – Yancey County Department of Social Services

III. Model Stormwater Ordinance

High Country COG has developed a model stormwater ordinance that can be used by local governments in the region. The model ordinance is intended to address development projects that involve less than one acre of disturbance. The purpose of the

model ordinance is to provide a mechanism to regulate stormwater in small municipalities that have limited capacity to review, permit, and inspect development.

Generally, the model ordinance presented in this project is based on UNC Environmental Finance Center's *Universal Stormwater Model Ordinance for North Carolina*. It removes much of the administrative functions (i.e., application review, permitting) from the Universal Ordinance, but sets standards for new development and requires new development projects to provide site plans prepared by a professional engineer that address post-construction stormwater management.

The model ordinance was provided to local governments in the region. Additionally, High Country COG staff presented the model ordinance to the Planning Boards of the Towns of Wilkesboro and North Wilkesboro. As of September 30, 2012, no local governments in the region have adopted a version of the model ordinance.

IV. Regional Mapping

The following GIS datasets were created to help guide local decisions on future stormwater BMP projects:

1. Erodible Soils – based on NRCS SSURGO soil tabular data. Data was extracted in Microsoft Access and imported in ArcGIS to map highly erodible soils based on the soil K factor. The K factor indicates erodibility based solely on surface texture. K factors greater than or equal to 0.24 were chosen based on academic articles to reflect erodible soils.
2. Steep Slopes – based on 2007 NC DOT LiDAR digital elevation data. Slopes were generated in percent rise and reclassified to only show slopes greater than 30%.
3. Highly Erodible Lands – retrieved from USDA NRCS Waynesville office. Depicts Highly Erodible Lands in our region based on soil texture and topographic characteristics, i.e. slope, length of slope, etc.
4. Regional Impervious Surfaces – retrieved 2006 National Land Cover Data and extracted to show impervious surfaces across the region.
5. Site Impervious Surfaces – digitized impervious surfaces for each BMP drainage area using 2010 orthophotography.
6. Hydrology – based on NC Stream Mapping Program data generated from LiDAR technology and aerial imagery resources. The data is unavailable for Wilkes County, therefore the NC Floodplain Mapping Program hydrology data was substituted.
7. Catchment Areas – Delineated sub-catchments for all 19 municipalities and surrounding areas based on NED 1/3 arcsecond (10m resolution) digital elevation model and NC Stream Mapping Program hydrology data. In Wilkes County, NC Floodplain Mapping Program hydrology data was used in the absence of NC Stream Mapping Program data. The Stream Definition tool was used to define smaller catchment areas. The sub-catchments were delineated in order to view

catchments at a smaller scale than the National Hydrologic Datasets' available catchment data.

8. BMP Drainage Areas – Created micro-level drainage areas specific to the proposed BMP sites based on field observation, delineated sub-catchments, hydrology and topographic digital data.
9. Public Property – Queried each county's tax parcel digital data to extract all county and town-owned parcels to help locate potential BMP sites.

High Country Council of Governments
Regional Stormwater Project
Alleghany County



Alleghany County EMS Station



Problem

The adjacent fire department and EMS properties are heavily paved to accommodate the emergency vehicles and thus generate a significant amount of runoff which flows offsite, causing erosion on a neighboring property. The site contributes to stormwater from elsewhere in town which discharges into a stream approximately 200 feet away. The runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 0.81 acre

Impervious surface = 0.67 acre; 83%

Affected stream = unnamed tributary to Bledsoe Creek

Stream classification = C, Tr



BMP solution

Much of the runoff can be captured at a point behind the EMS building, held, and filtered with a bioretention cell. This BMP was selected based on site characteristics, space limitation, and the desire of the building occupants to maintain a landscaped appearance. There is approximately 600 square feet of space available for the bioretention cell.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will help lessen erosion downstream that often results from higher stream volumes. The site is located in Sparta where the Town government has recently completed a number of significant stormwater retrofit projects, with more planned. This BMP will augment those projects and contribute to the overall reduction of runoff throughout town.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	17		5		13
COD	300		U		U
TSS	771		108		663
LEAD	1		1		0
COPPER	0		U		U
ZINC	1		1		0
TDS	2,068		U		U
TN	4		2		2
TKN	6		U		U
DP	0		U		U
TP	1		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$40,795
Design & engineering	<u>2,039</u>
	\$42,834

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Project Program
North Carolina Division of Water Resources Development Project Grant Program

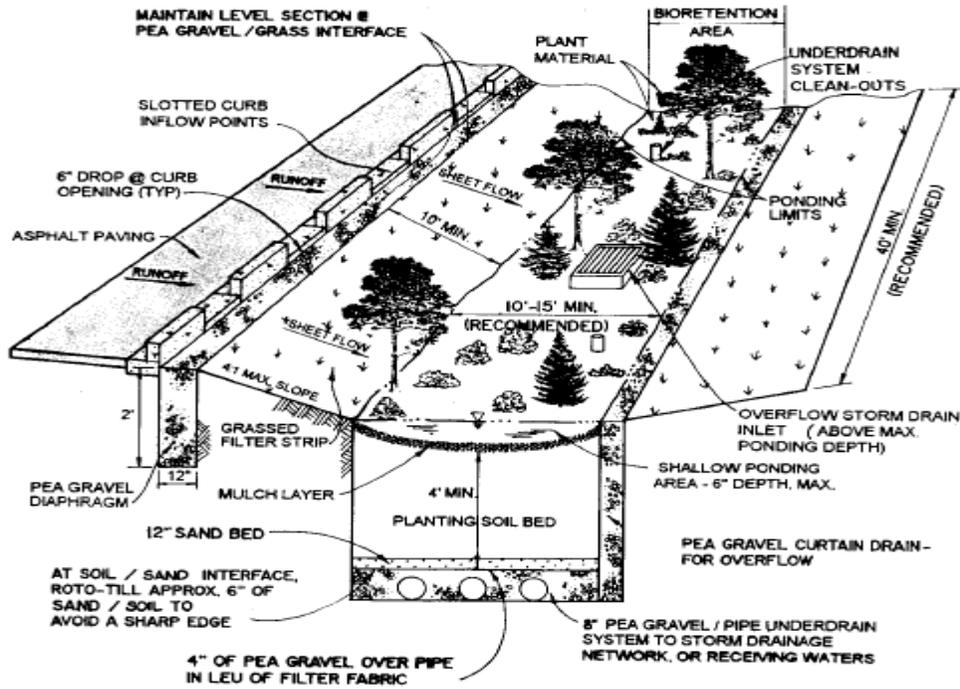
¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

**Allegheny County Site:
Allegheny County
EMS Station**



Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Sparta



Town Maintenance Facility



Problem

Impervious surfaces at the Town maintenance facility results in a substantial amount of uncontrolled runoff. The runoff from the site contains oils and grease, hydrocarbons, metals, and road salt that drains to neighboring properties and ultimately to a stream located approximately 600 feet from the site.

Drainage area = 1.86 acre
Impervious surface = 1.38 acre; 74%

Affected stream = unnamed tributary to Bledsoe Creek

Stream classification = C, Tr



BMP solution

The only available space for BMP treatment is the two grassy strips flanking the entrance drive. Their shapes and dimensions dictate that vegetated swales will be the BMP used. A grade-level drain will be necessary to direct the runoff to the swales.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the nearby stream. The Town is aware of the need for stormwater management and recently completed a number of significant retrofit projects, with more planned. This BMP will augment those projects and contribute to the overall reduction of runoff throughout town.

Vegetated swale	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	56		28		28
COD	963		578		385
TSS	2,474		668		1,806
LEAD	3		2		1
COPPER	1		U		U
ZINC	4		1		2
TDS	6,631		U		U
TN	14		9		6
TKN	20		U		U
DP	0		U		U
TP	2		1		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$25,047
Design & engineering	<u>2,500</u>
	\$27,547

Funding

NC Clean Water Management Trust Fund
 Soil & Water District Community Conservation Assistance Program
 North Carolina Division of Water Resources Development Project Grant Program

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Town of Sparta Site: Town Maintenance Facility

MEMORIAL PARK DR

SWAIN ST

SPARTA TOWN HALL

TOWN OF SPARTA

ALLEGHANY COUNTY

JONES ST

Site 1: Vegetated Swale

Site 2: Vegetated Swale

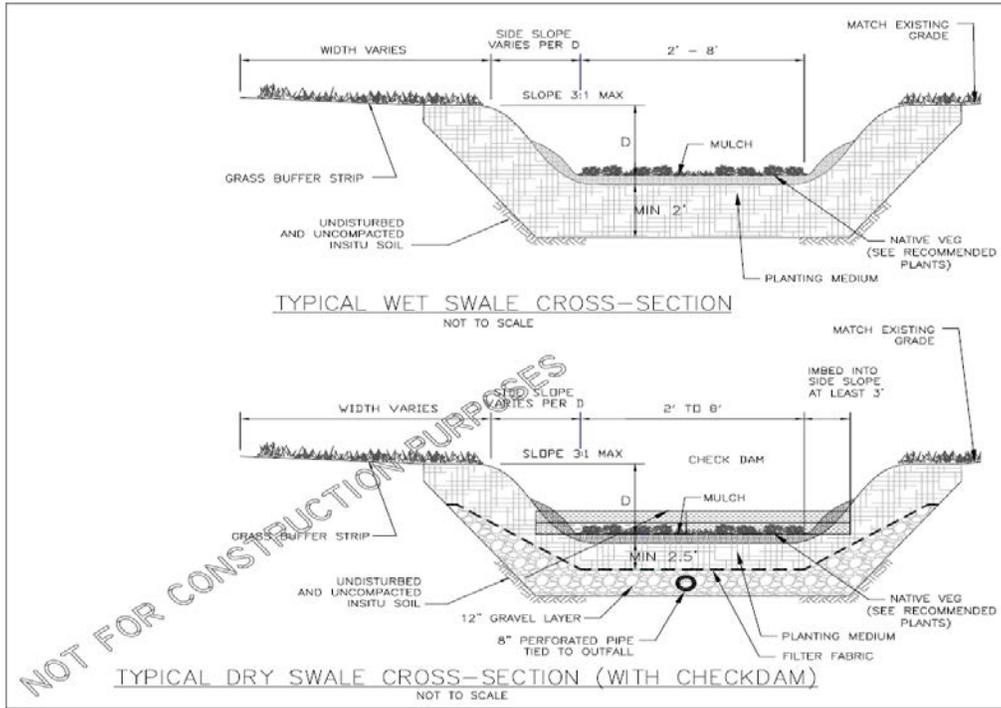
Legend:

- Parcels (white outline)
- Drainage Area (red outline)
- Roads (black line)
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert (yellow line)
- Stream / River (blue line)
- 2010 Orthophotography

1 inch = 65 feet

0 30 60 Feet

Diagram and photograph of a typical swale:



High Country Council of Governments
Regional Stormwater Project
Ashe County



Ashe County Courthouse



Problem

Runoff from parking lots (one complete; one under construction) is piped below the road and exits below a retaining wall as shown in the photos. Runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to an intermittent stream channel. During extremely heavy rains, the stormwater contributes to flooding in the highway below and enters Naked Creek. In addition to existing stormwater concerns, future development planned for the area below the discharge will require management of stormwater from this site.

Drainage area = 11.72 acres
Impervious area = 2.29 acres; 20%
Affected stream = unnamed tributary to Naked Creek
Stream classification = C +



BMP solution

A detention basin will be constructed below the discharge point in the retaining wall. The site will not interfere with planned development elsewhere at this location. The BMP was selected based on the volume of stormwater, the availability of space, and the inconspicuous location.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. Temperature fluctuations caused by this site's runoff will be eliminated.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	69		50		19
COD	1,186		949		237
TSS	3,046		1,294		1,751
LEAD	4		2		2
COPPER	1		U		U
ZINC	4		3		1
TDS	8,164		U		U
TN	18		12		5
TKN	25		U		U
DP	0		U		U
TP	3		2		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$17,175
Engineering	<u>1,000</u>
	\$18,175

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

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Ashe County Site: Ashe County Courthouse

Site 1: Detention Basin

ASHE COUNTY

GOVERNMENT CIR

ASHE COUNTY
COURTHOUSE

LAUREL LN

HILLSIDE LN

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 200 feet

0 100 200 Feet

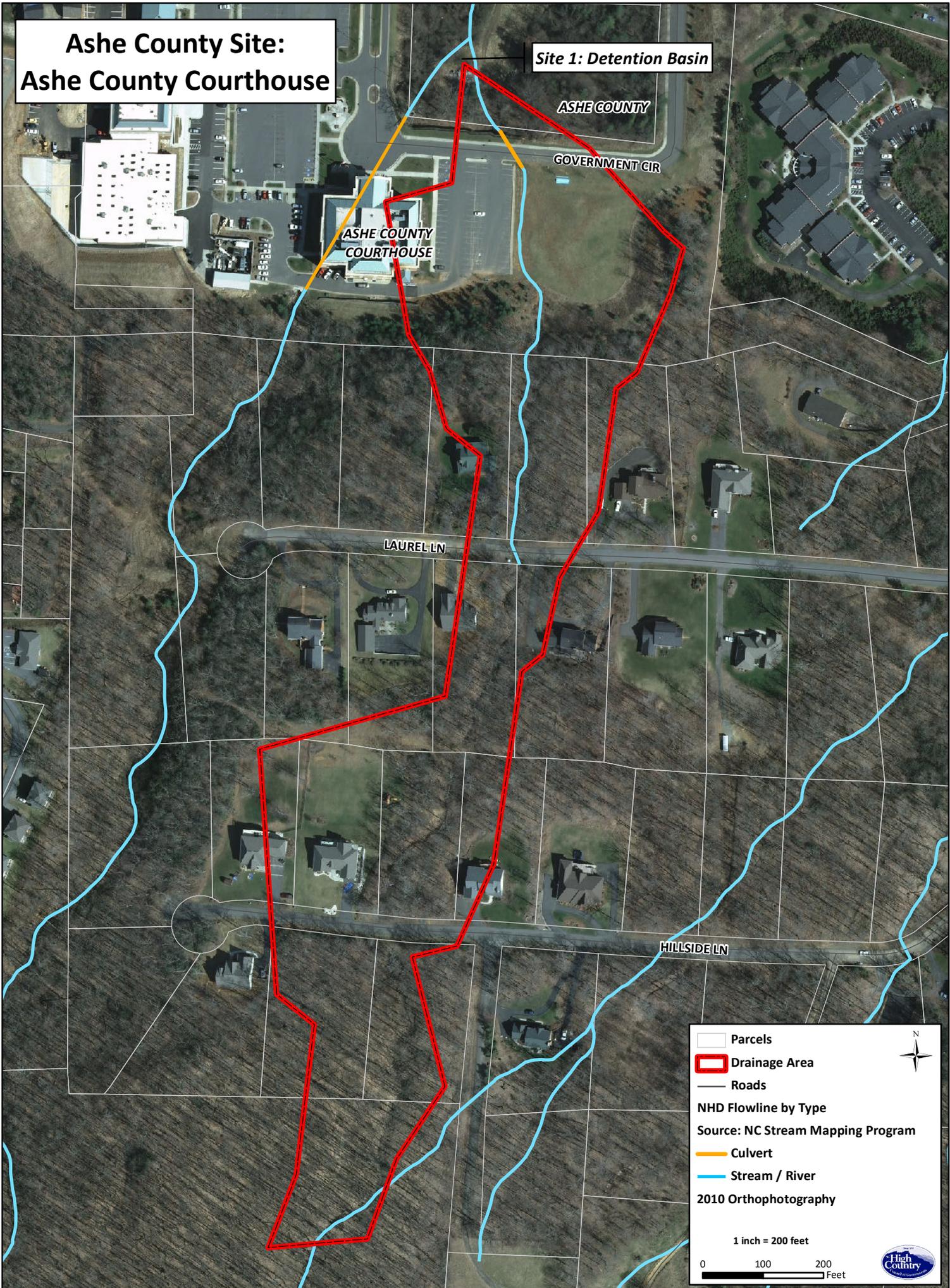
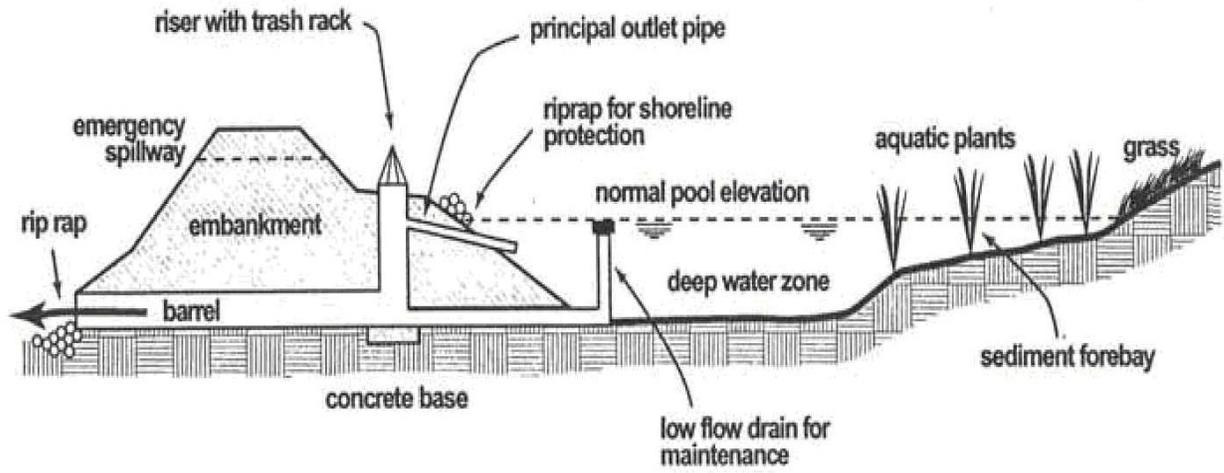


Diagram and photograph of a typical detention basin:



High Country Council of Governments
Regional Stormwater Project
Town of Jefferson



Ashe Memorial Hospital



Problem

The hospital is located on top of a hill. Much of the runoff from the hospital building, parking, and roads collects at a single culvert which drains directly into a stream (red arrow indicates location of discharge). The runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to the receiving stream.



Drainage area = 4.93 acres
Impervious surface = 3.20 acres; 65%

Affected stream = unnamed tributary to
Naked Creek
Stream classification = C +

BMP solution

The stormwater enters the stream via a culvert in the wooded area (blue arrow). The stream flows out of the woods into the unmowed area on the left. Because the site is not visible from the hospital campus and abundant space is available, stormwater will be diverted via a flowsplitter into an extended linear detention basin that will be constructed parallel to the stream.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. Temperature fluctuations caused by this site’s runoff will be eliminated.

Detention basin	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	148		41		106
COD	2,554		U		U
TSS	6,557		918		5,639
LEAD	8		5		3
COPPER	2		U		U
ZINC	9		7		2
TDS	17,575		U		U
TN	38		17		21
TKN	54		U		U
DP	0		U		U
TP	5		2		4
CADMIUM	0		U		U

Cost estimate²

Construction, including flowsplitter \$35,325
 Design & engineering 7,065
 \$42,390

Funding

NC Clean Water Management Trust Fund

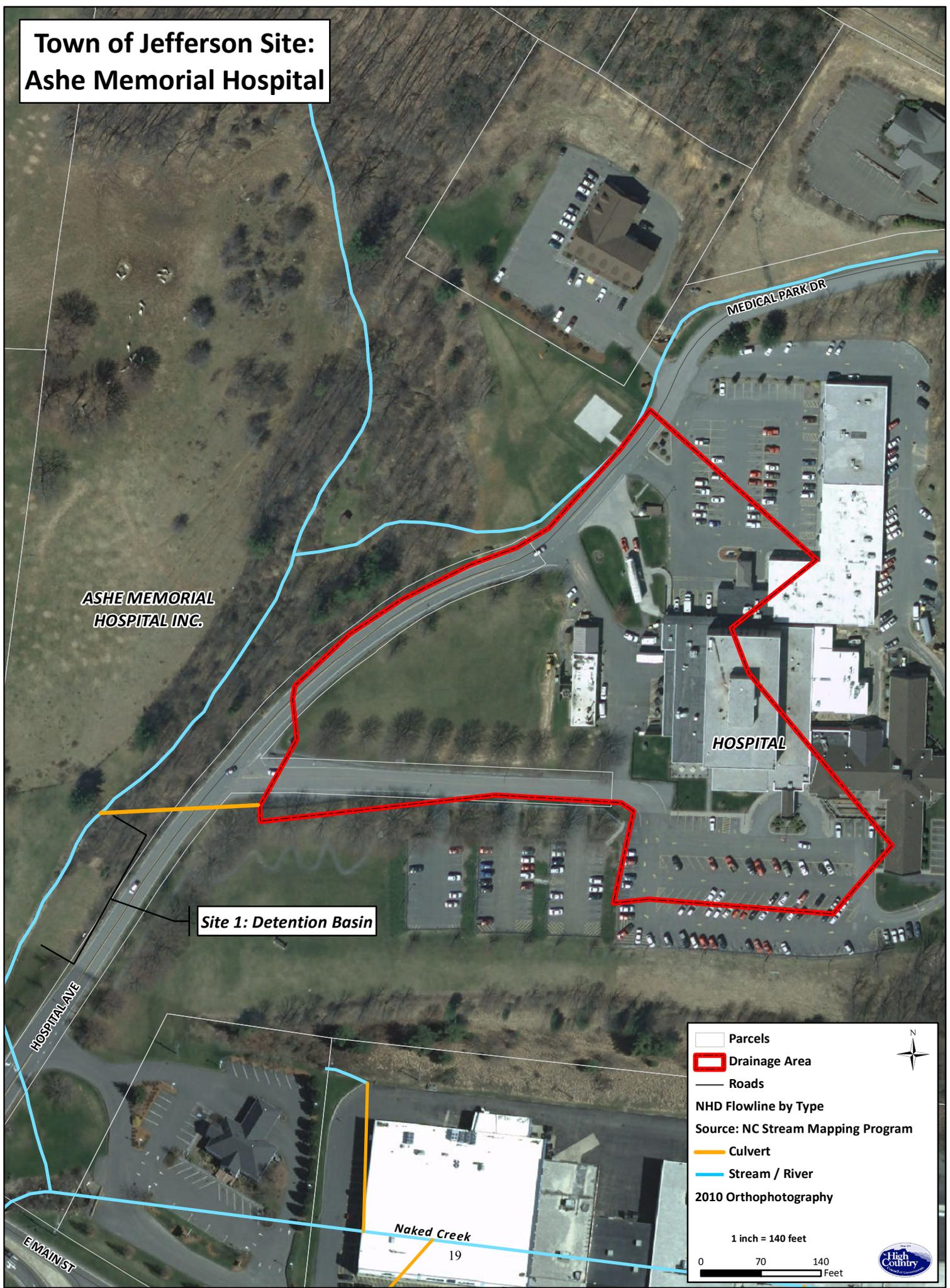
 Soil & Water District Community Conservation Assistance Program

 North Carolina Division of Water Resources Development Project Grant Program

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Town of Jefferson Site: Ashe Memorial Hospital



ASHE MEMORIAL
HOSPITAL INC.

MEDICAL PARK DR

HOSPITAL

Site 1: Detention Basin

HOSPITAL AVE

Naked Creek

19

E MAIN ST

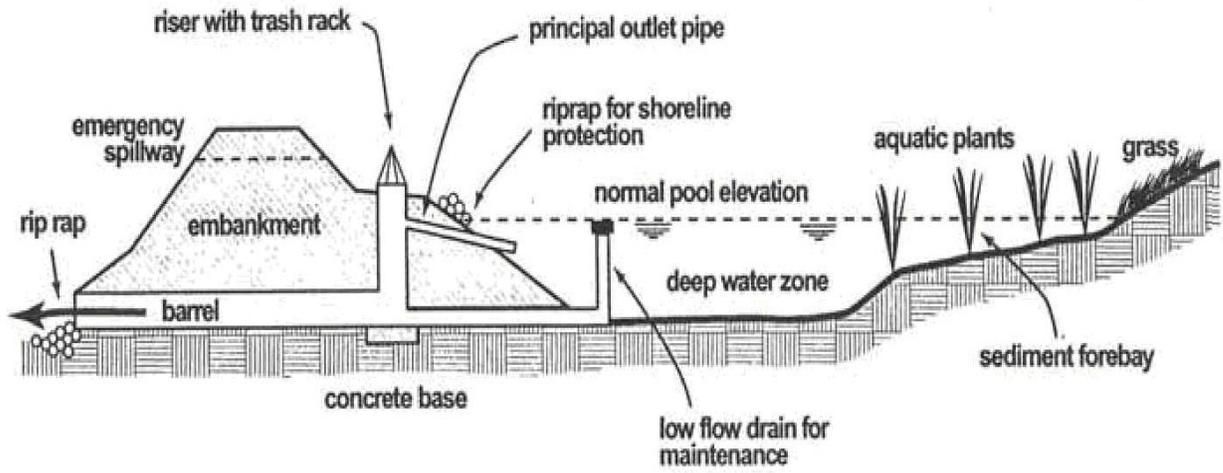
Legend:

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 140 feet

0 70 140 Feet

Diagram and photograph of a typical detention basin:





Town of Jefferson Jefferson Fire & Gates Corporation

Location Northwest Drive off Hwy 88 in Jefferson

Landowner Project activities will occur on two tracts, separately owned by the Town of Jefferson and Ashe County Job Development, Inc. The Ashe County tract is leased to Gates Corporation.

468 New Market Blvd
Boone, North Carolina 28607
828-265-5434 ext 114
828-265-5439 fax
Kelly Coffey
kcoffey@regiond.org

Affected stream unnamed tributary; New River basin

Problem This site contains two problem areas exhibiting erosion and impervious surface runoff. The proposed solution aims to address these concerns by developing each site individually and attempting treat the stormwater in a piecemeal approach rather than with an end-of-the-line, large-scale solution. Site 1 consists of the eroding drainage ditch on the eastern side of Northwest Dr. between the road and the fence (Photo Page; Photo 2). Currently the ditch receives runoff from a culvert running under Gates Ln. and discharges through an outlet culvert which passes underneath Northwest Dr. The ditch is currently experiencing minor erosion along its banks. The ditch can be developed into a stormwater BMP to treat the runoff passing through. At Site 2 a stream passes through a culvert under the fire department parking lot and then flows in an open channel (Photo 1) approximately 25 feet before flowing under Northwest Drive. The open channel on the western side of Northwest Dr. receives stormwater from both Site 1 (Photo 3) and surface flow from Northwest Drive and the fire department parking lot (Photos 1 & 4). As a result, the open channel is experiencing significant erosion along the banks. Likewise, the site also has space to introduce a BMP to treat the incoming stormwater.

Proposed structure	Site 1 vegetated swale w/ check-dams	Site 2 streambank restoration
Cost	\$10,106	\$4,211

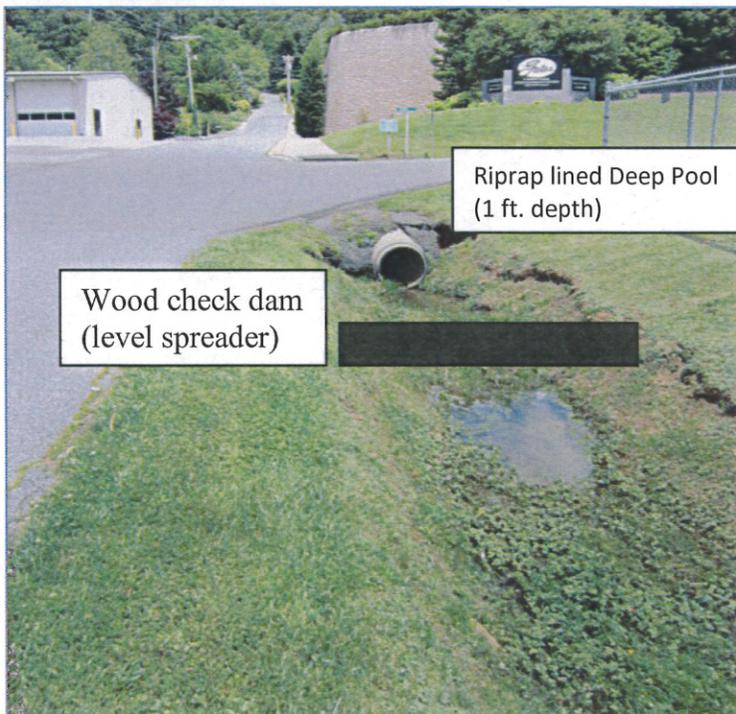
Water quality objective reduction of pollutants

Anticipated result reduction in nitrogen (20%)
phosphorus (20%)
suspended solids (35%)

Drainage area 10.5 acres

Impervious area 5.9 acres or 56% of drainage area

Solution description

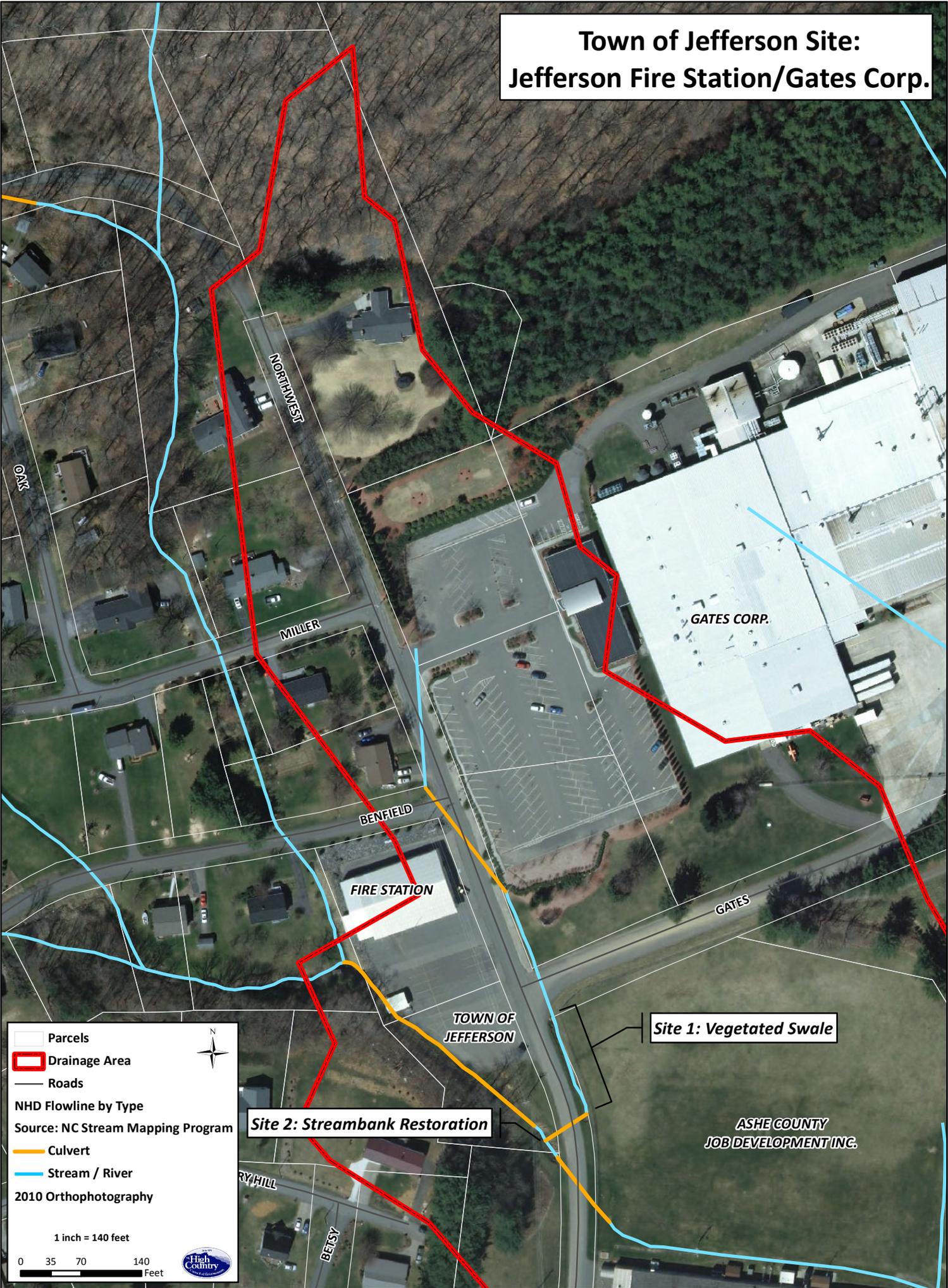


SITE 1

Site 1 will be developed into a vegetated swale with intermediate check dams to both slow velocity and treat incoming stormwater. The lower velocity will prevent bank erosion and allow sediment to fall out of suspension in the stormwater, thus decreasing the TSS (total suspended solids) of the stormwater effluent. This treatment is accomplished by two facets of the vegetated swale design: enhanced vegetation and intermediate check dams. Taller grass in the channel will act as an energy dissipater for the passing water, slowing it as it moves from inlet to outlet. The grass will also allow greater infiltration by promoting soil channels as it roots and naturally filter pollutants in the runoff through phytoremediation. The check dams additionally slow water velocity and increase the retention time of the water in the BMP. Increased retention time allows the soil, plants, and bacteria additional time to biodegrade undesirable pollutants in the runoff such as TP, TN, metals, and other substances.

Directly below the inlet culvert a deep pool will be established, roughly one foot deep and two feet long. This deep pool will be lined with riprap to dissipate the energy of water exiting the culvert and prevent erosion. At the

Town of Jefferson Site: Jefferson Fire Station/Gates Corp.



Parcels
Drainage Area
Roads
NHD Flowline by Type
Source: NC Stream Mapping Program
Culvert
Stream / River
2010 Orthophotography

1 inch = 140 feet
0 35 70 140 Feet

Site 2: Streambank Restoration

Site 1: Vegetated Swale

*ASHE COUNTY
JOB DEVELOPMENT INC.*

Photo 1



Photo Page

Culvert carrying flow from ditch



Photo 2



Photo 3



Photo 4



Photo 5



downstream edge of this deep pool area a wood check dam will be constructed out of treated 2x8. The top of this initial check dam will not rise above the inlet culvert and serves as a level spreader for water entering the channel. Following this wood check dam will be a series of 4 check dams (one dam approximately every 30 ft. of channel) constructed of rip rap. Rip rap was selected as a check dam material because of its resistance to erosion. Other suitable materials may also be used, such as river rock or other aesthetic alternatives. Earthen check dams typically erode over time with heavy storm events and would increase site maintenance costs. The final check dam will be located roughly 4-5 feet from the outlet culvert. The entrance to the outlet culvert will also be excavated to a depth of one foot and a deep pool constructed to allow greater retention time and slow velocity sharply. This particular area is experiencing the greatest amount of erosion. The top elevation of any of the rip rap check dams should not come within 3-4" of the top of the channel to prevent stormwater from leaving the channel and entering the road.

Between the wood check dam and the final rip rap dam the entire channel will be planted with a close-growth, water-tolerant grass. This grass will need to be allowed to grow to an adequate height, preferably between 12-18". Successful swale grasses include red fescue, Kentucky bluegrass, and reed canary; however a local landscape professional should be consulted to ensure successful growth. If a grass is seeded that is not particularly suited to long periods of soil saturation the vegetative growth may die and the channel will lose its treatment efficiency. A landscape professional may be able to help develop a more aesthetic planting design if vegetation other than grass is desired. Trees and shrubs, however, are not recommended. The banks of the channel should be seeded or allowed to grow grass naturally. Spraying of the banks to kill growth should be avoided as it decreases the stability of the channel banks.

Vegetated swale similar to the recommended solution for Site 1. The grassed section in the background & the ornamental plantings in the foreground illustrate the available options.



Pollutant Loading

An analysis of the incoming runoff from the surrounding area was done using the SCS Curve Number Method in conjunction with Schuler’s Simple Method. Estimates of pollutant loading in this runoff show 150 lb. TSS, 1 lb. TP, and 6 lb. TN annually. Sediment, in addition to the suspended solids in the runoff, is lost from bank erosion due to high velocity runoff. The grass swale with check dams and forebays addresses this problem by slowing runoff velocity and allowing sediment to settle out. NCDENR projects grassed swales to have removal efficiencies of 35% TSS, 20% TN, and 20% TP. This is an accurate estimate for this design, particularly considering the effect the check dams and forebays will have on runoff velocity. Several studies have also shown that grassed swales and vegetative waterways have moderate efficiency in removing heavy metals from stormwater. Runoff from the parking area, nearby roofs, and roads carries copper, lead, and zinc into the drainage ditch.

Results

	total nitrogen	total phosphorus	total suspended solids
annual pollutant load (lbs)	6	1	150
wetland removal %	20%	20%	35%
net pollutant reduction (lbs)	1.2	0.2	52.5

Site 1 Cost

\$84.22 per linear foot (EPA-estimated cost for vegetated swales) x 120' = \$10,106

SITE 2

Site 2 at this location is the open stream channel to the west of Northwest Dr. It receives overland flow from the Jefferson Firehouse parking lot and Northwest Dr., and an inlet culvert draining water from the upper drainage ditch to the east of Northwest Dr. (Site 1).

The streambank is experiencing significant erosion from high velocity runoff along the upper and right banks. High velocity of the stormwater is due to the unimpeded flow of water across the parking lot and the steeply sloping Northwest Dr. which directs water towards the ditch. A Bank Erosion Hazard Index (BEHI) evaluation results in a “High” erodibility rating. Bank erosion is of concern not simply for the amount of suspended solids it introduces to the stormwater runoff; current erosion patterns indicate the bank will

continue to lose sediment in the direction of Northwest Dr. This will lead to cracking, depression formation, and potentially failure of the asphalt if left unchecked. Additionally, runoff is entering the stream with little filtration.

The steep grades of the firehouse parking lot and Northwest Dr. move runoff rapidly across the impervious asphalt, generating high velocity. Current maintenance of the site involves mowing the grass uphill and to the right of the drainage ditch. During large rain events this allows water to move relatively unobstructed when soil conditions are saturated. Water can be slowed down before moving down the ditch slope by simply allowing established grasses to reach a taller height. Current maintenance of the site should be altered to allow the grass surrounding the channel to reach a height of 18-24". This will create a vegetative buffer between the asphalt and the channel, reducing the velocity of the stormwater. Interspersed shrubs contained within the grassy area can give a more aesthetic alternative to simple grassed waterway. An ornamental border grass such as monkey grass is another alternative that would give the site a landscaped look while filtering the runoff. It appears that herbicide has been applied to the banks to control grass growth. Not only is this practice harmful for the chemicals it introduces to the immediate stream but it prohibits necessary vegetation that helps stabilize the channel banks.

The streambank can be stabilized on the existing grade of the bank and erosion eliminated by establishing vegetation. The bank will be seeded with reed canary grass, or available alternative with dense growth. After seeding and planting is completed the vegetated area should be covered with mulch and an erosion blanket until vegetation is established. An initial fertilization of the bank along with a brief period of watering may be necessary to ensure plant growth. A landscape professional should be consulted regarding appropriate vegetation, planting schedule, soil mix, fertilization if necessary, and other possible concerns such as aesthetics.

Pollutant Loading

Pollutant loading calculations were done to determine if the site uphill of the drainage ditch would be a viable location for a bioretention cell. This site would treat only the runoff from the firehouse parking lot. Although the location would be ideal for a bioretention cell, pollutant estimates show that because the area is so small the construction costs outweigh the potential benefits. Less than a 1 lb. of TN and TP would be carried in runoff annually from the site. TSS loading is 11.1 lb/year from the parking lot runoff. A major amount of sediment is exported from the site annually by the continuous erosion of the streambanks. This maintenance and landscape plan will help to stop the excessive sediment removal from the site by stabilizing the soil and slowing nearby runoff velocity. Several studies have also shown that grassed swales and vegetative waterways have moderate efficiency in removing heavy metals from stormwater. Runoff from the parking area carries copper, lead, and zinc into the stream. The vegetative filter strip above the open channel will be effective but will have a lower efficiency for heavy metal treatment than the upper grassed swale due to a decreased retention time.

Site 2 Cost

\$84.22 per linear foot (EPA-estimated cost for vegetated swales) x 50' = \$4,211

Total Costs

\$84.22 per linear foot (EPA-estimated cost for vegetated swales) x 120' =	\$10,106	Site 1
\$84.22 per linear foot (EPA-estimated cost for vegetated swales) x 50' =	<u>\$ 4,211</u>	Site 2
	\$14,317	Total cost for location

Permitting

Due to the low development of this design, minimal grading, and no clearing; no permitting will be required. Proper erosion control methods should be observed when grading to prevent sediment pollution and soil loss.

Maintenance

After the site is landscaped and constructed as described, several current maintenance patterns would need to be changed. Spraying of the banks to kill vegetation in either drainage ditch is strongly discouraged. Vegetation will help to stabilize the slopes from high velocity runoff and removing this vegetation will continue the current soil loss trend.

Grass above the stream channel should be maintained at a height of 18-24" or higher in order to slow runoff coming from the firehouse parking lot and provide some filtration for the water. Grass alongside the channel next to Northwest Dr. should also be kept at this height as runoff coming down the road and spilling over the sides of the channel is causing erosion. The streambank should be monitored biweekly for several months after initial construction to verify that vegetation has established. The site should be inspected over the course of a year to detect other possible concerns and determine if erosion has been brought under control.

Height of the grass within the upper drainage ditch should be kept at a height of 12-18". This includes the ditch bottom as well as the side slopes. Grass height alongside the ditch does not impact the effectiveness of the swale or seems to be affecting erosion so the

landscaper may choose a maintenance height. Rip rap check dams should be checked periodically for debris and for any signs of failure. The area should also be observed, if possible, during a significant rain event; if the surface of the check dams are too close to the top of the ditch and are causing flooding due to constricted flow, dam height will need to be adjusted. Since the forebays are responsible for causing sedimentation, these areas need to be checked regularly and cleared of sediment. Failure of forebays to remove sediment from runoff is typically due to poor maintenance and loss of the pooling area due to sediment collection. The site should be checked periodically for erosion, debris, and vegetative establishment.

Possible funding sources

North Carolina Clean Water Management Trust Fund

Application due date: February 1

Eligible expenses: planning, designing and constructing a project, including excess or reserve capacity costs; legal, fiscal administrative and contingency costs; fees to obtain a loan or grant; permit fees; and property acquisition costs.

Maximum: \$3.0 million per grant recipient

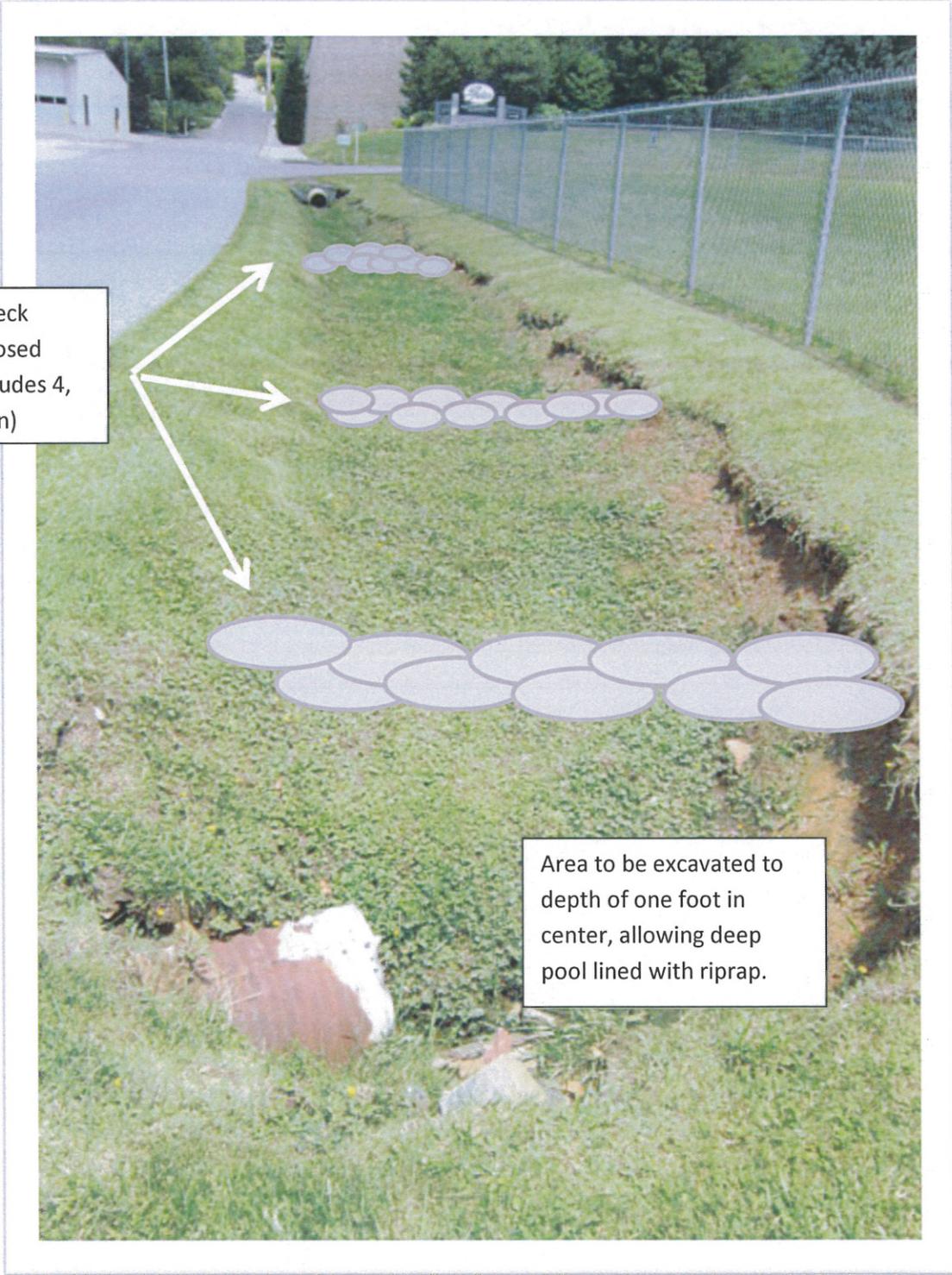
Match: 20% required but can be waived; level of match affects the project's score

Other requirements: permanent conservation agreement with the landowner; 10-year maintenance

Community Conservation Assistance Program- Soil & Water Conservation Districts

- Individual counties usually receive less than \$5,000 annually based on a variety of funding factors.
- Funds up to 75% of the cost of the BMP
- No strict deadline, but funds usually are encumbered by March

Gates Corporation

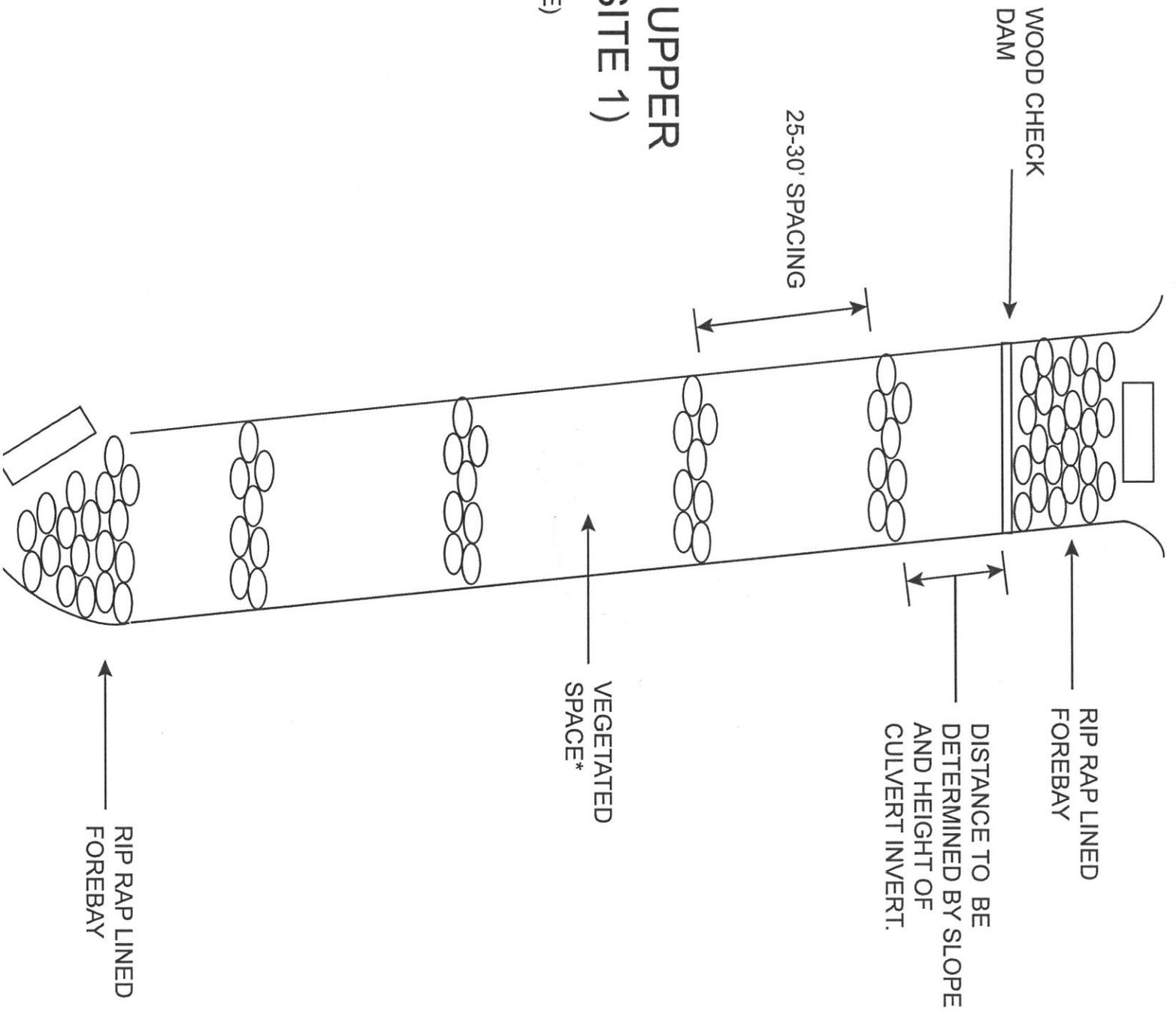


Series of Check Dams (proposed solution includes 4, only 3 shown)

Area to be excavated to depth of one foot in center, allowing deep pool lined with riprap.

OVERHEAD VIEW OF UPPER DRAINAGE DITCH (SITE 1)

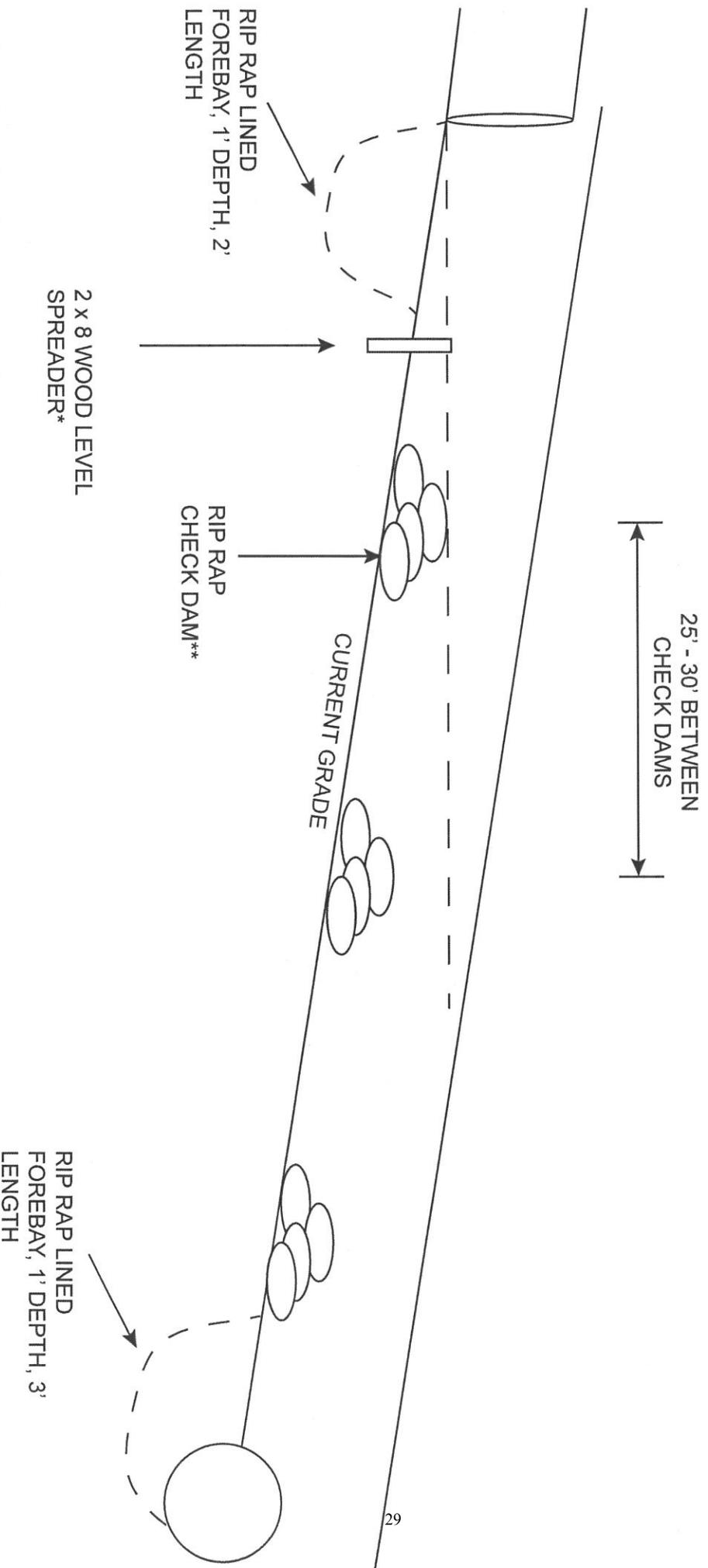
(DRAWING NOT TO SCALE)



*Spaces between check dams shall be seeded with an appropriate grass mixture and allowed to grow to 12-18" in height. Vegetated space includes ditch and banks.

PROFILE VIEW OF UPPER DRAINAGE DITCH (SITE 1)

(DRAWING NOT TO SCALE)

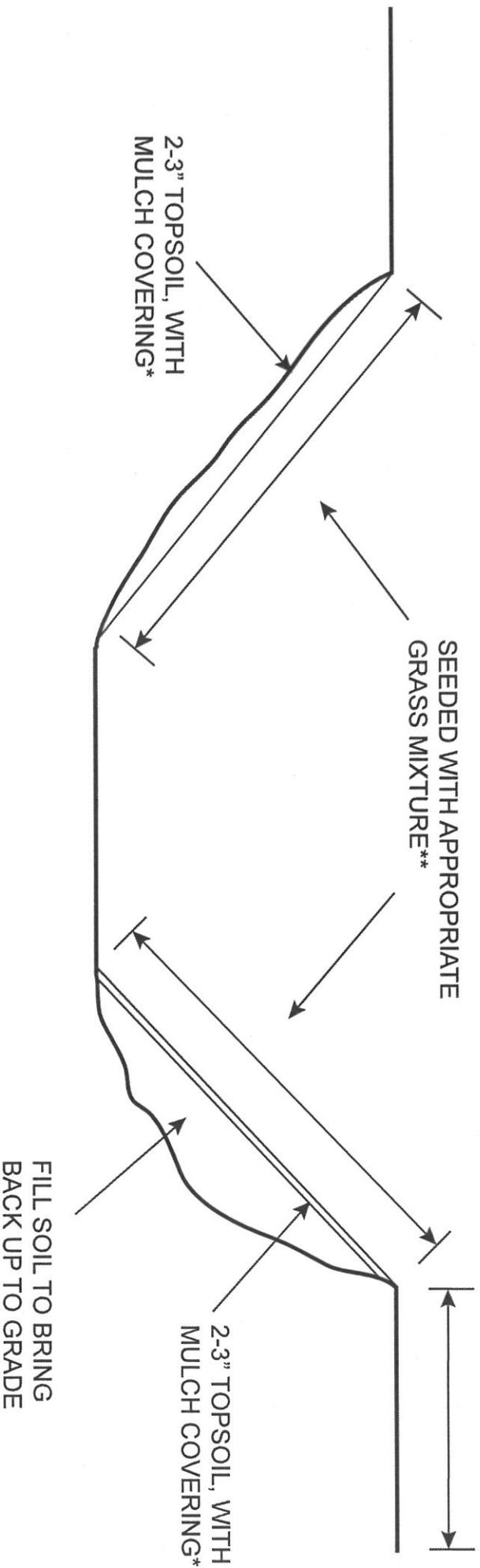


- * Top of wood level spreader shall not be more than 1" above inlet invert. Shall be constructed of treated 2 x 8, if necessary held in place by supporting 4 x 4 or 2 x 4 or rip rap brace.
- ** Top of first check dam shall not be above the invert inlet. Check dams should be constructed of graded rip rap, river rock, or equivalent.

PROFILE VIEW OF LOWER DRAINAGE DITCH (SITE 2)

(DRAWING NOT TO SCALE)

GRASS ALONG
DITCH CREST TO BE
MAINTAINED
BETWEEN 12-18"



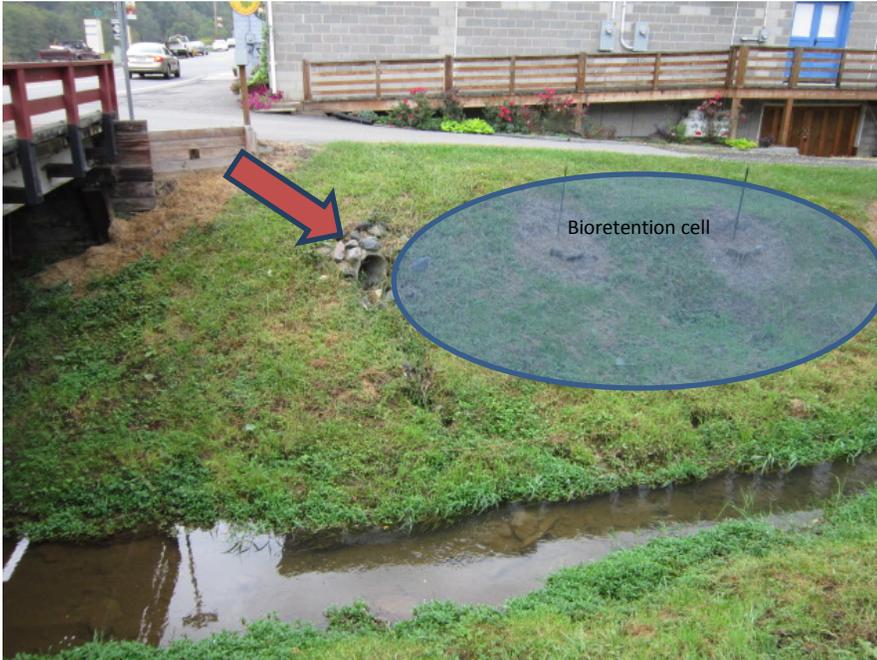
*If an erosion blanket is recommended by the landscaper, one should be used and placed over top of mulch to prevent erosion during vegetative establishment.
**Reed canary grass or kentucky bluegrass are good seeding mixtures. Landscaper may be helpful in recommending other aesthetic and appropriate alternatives.

High Country Council of Governments
Regional Stormwater Project
Town of Lansing



Bridge at Old Field Branch

Problem



Runoff from the street is piped directly into the receiving stream. The runoff from the pavement introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 0.12 acre
Impervious surface = 0.11; 94%

Affected stream = Old Field Branch
Stream classification = C, Tr, HQW



BMP solution

A bioretention cell will be installed at the discharge point to capture and filter the runoff. The inclusion of appropriate plants will enhance its filtering capacity while maintaining the manicured appearance of the site. The slope and width of the area available will limit the size of the bioretention cell.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that often results from higher stream volumes.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	4		1		3
COD	62		U		U
TSS	160		10		150
LEAD	0		U		U
COPPER	0		U		U
ZINC	0		U		U
TDS	428		U		U
TN	1		U		U
TKN	1		U		U
DP	0		U		U
TP	0		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$2,794
Design & engineering	<u>500</u>
	\$3,294

Funding

North Carolina Clean Water Management Trust Fund

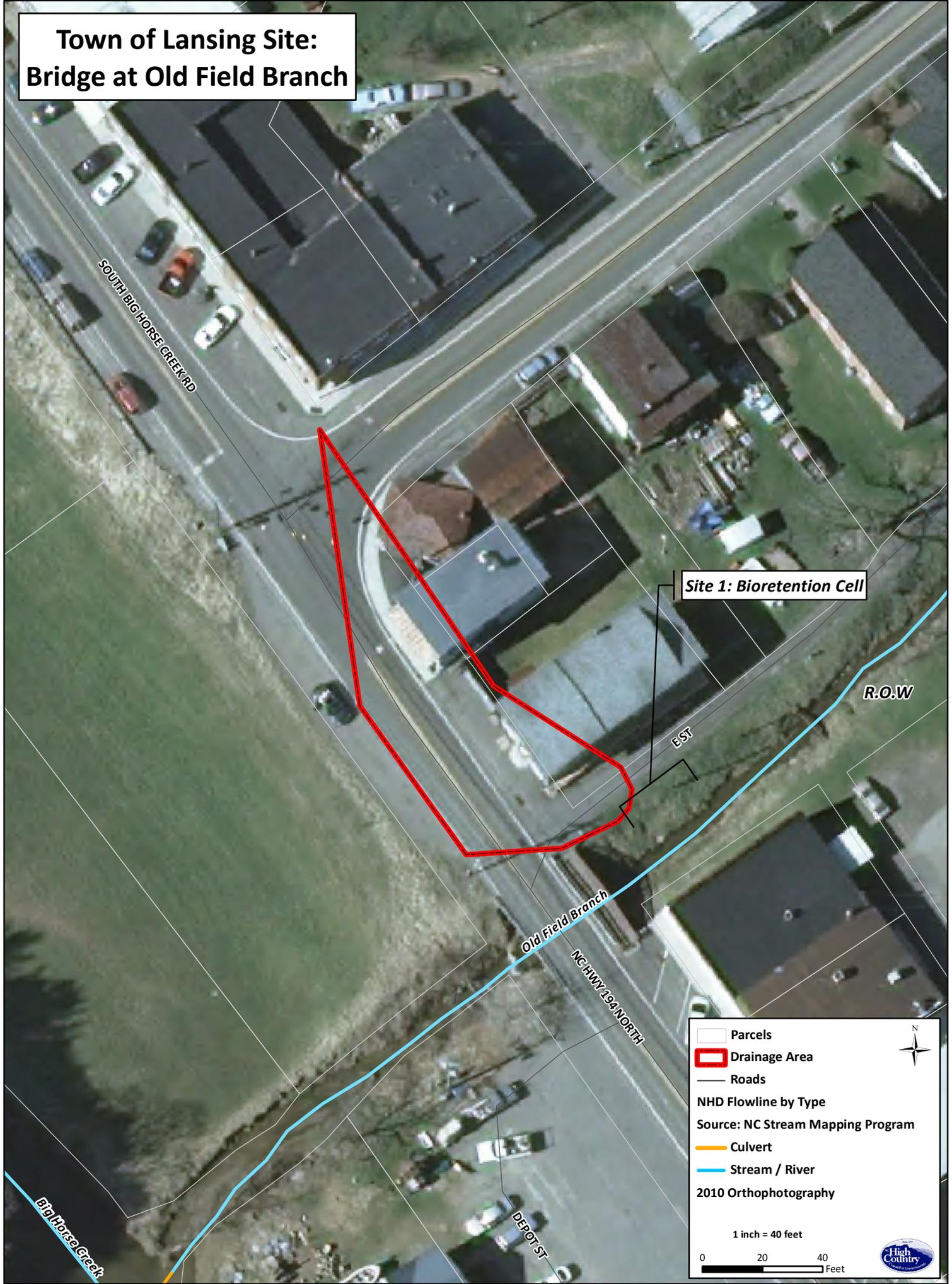
Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Lansing Site: Bridge at Old Field Branch



Site 1: Bioretention Cell

Parcels
Drainage Area
Roads
NHD Flowline by Type
Source: NC Stream Mapping Program
Culvert
Stream / River
2010 Orthophotography

1 inch = 40 feet

0 20 40 Feet

High Country
A Division of



High Country Council of Governments
 Regional Stormwater Project
 Town of West Jefferson



Bowie-Seagraves Park



Problem

A stream flows under the parking lot via a culvert and discharges into the park. Runoff from the parking lot and street is channeled and drained to the stream prior to its flow out of the culvert. Because the runoff adds considerable volume to the stream, it is contributing to streambank erosion in the park.

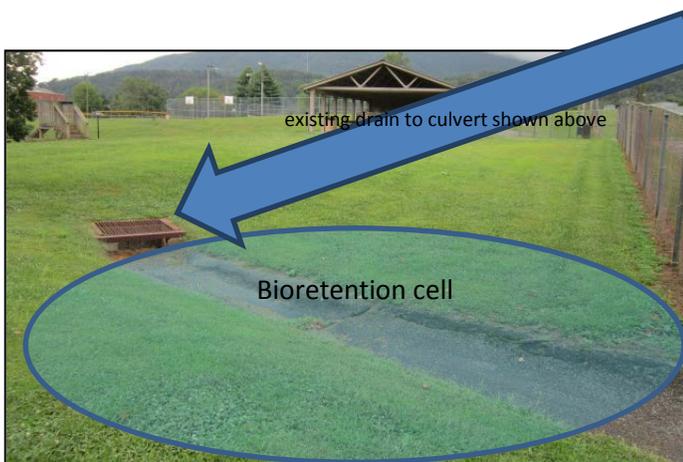


Drainage area = 6.47 acres
 Impervious surfaces = 2.05 acres; 32%

Affected stream = unnamed tributary to Little Buffalo Creek
 Stream classification = C, Tr +

BMP solution

A bioretention cell will be installed where the paved channel is located, with the existing drain used to capture overflow from extreme rain events. Bioretention with appropriate plants and landscaping is the best solution due to the need to maintain the park environment. With interpretive signage the BMP will also serve an educational purpose.



Water quality benefits

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. A lower stream volume during heavy rains will help reduce the amount of streambank erosion and sedimentation now occurring.

bioretention ¹	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	194		96		98
COD	3,351		2,011		1,341
TSS	8,605		2,323		6,282
LEAD	10		6		5
COPPER	2		U		U
ZINC	12		5		7
TDS	23,066		U		U
TN	50		30		20
TKN	71		U		U
DP	1		U		U
TP	7		4		3
CADMIUM	0		U		U

Cost estimate²

Construction	\$40,795
Design & engineering	<u>2,039</u>
	\$42,834

Funding

North Carolina Clean Water Management Trust Fund
 Soil & Water District Community Conservation Assistance Project Program
 North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of West Jefferson: Bowie Seagraves Park

FRANK ST

TOWN OF WEST JEFFERSON

Site 1: Bioretention Cell

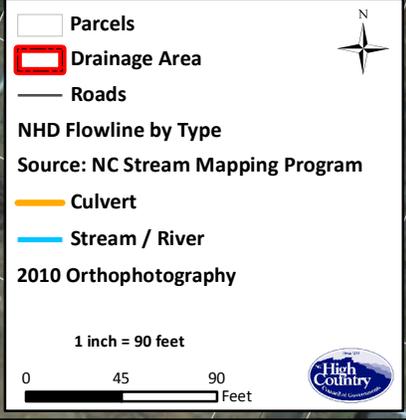
PARK RD

SOUTH CHURCH AVE

MOUNTAIN AVE

LONG ST

RIDGECREST AVE



Legend:

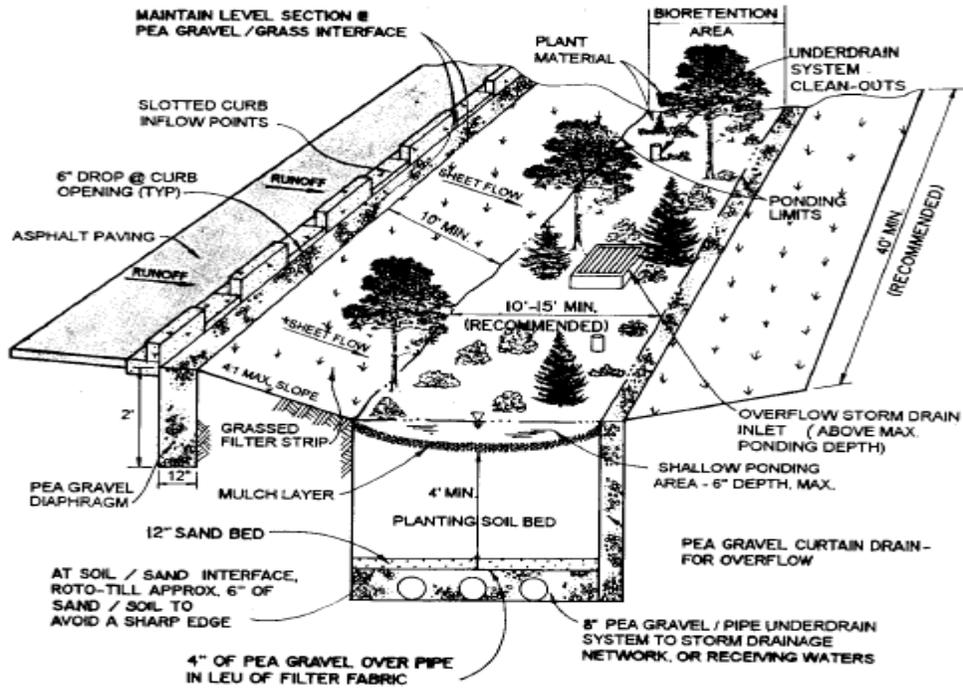
- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 90 feet

0 45 90 Feet



Diagram and photograph of a typical bioretention cell:

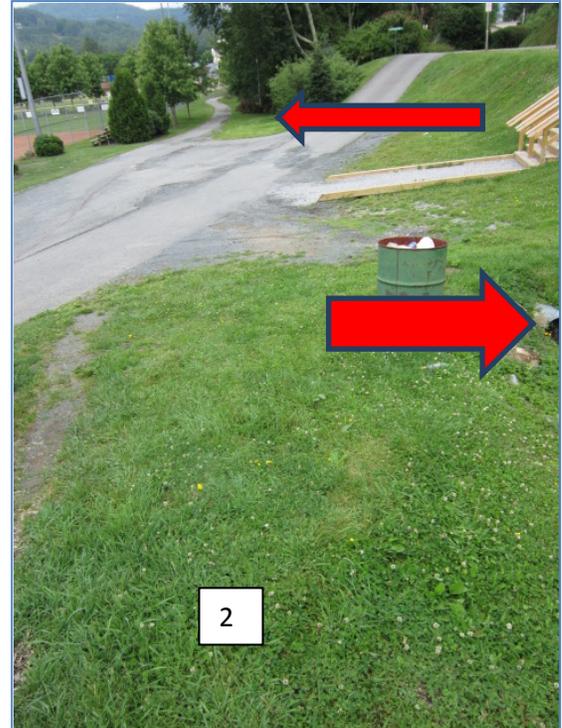


High Country Council of Governments
Regional Stormwater Project
Avery County



Banner Cabinets/Avery County Ballfield

Problem



The culvert on the right in Photo 1 carries a small stream and also captures runoff from the road. The culvert on the left drains the area around a cabinet manufacturer. Both culverts flow into a single culvert shown on the right in Photo 2, which then transports the stream and storm runoff to the edge of the wooded area in the background (other end shown in Photo 3). Surface runoff from pavement and surrounding structures contributes to stormflow in the same area. (3) The stream/runoff flows on the surface parallel to the walking trail and enters a drain pipe (Photo 4). The pipe cannot handle the storm flow, and is clogged with sediment.

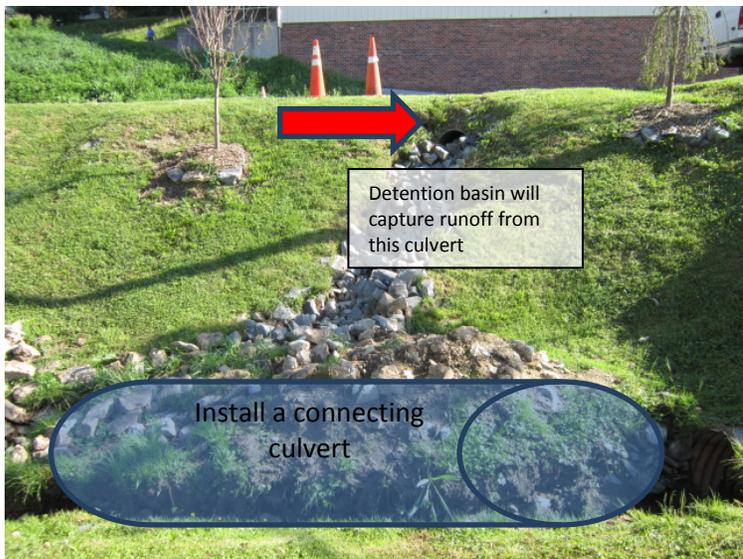




The water that is able to enter the pipe empties into the creek, contributing to flooding in this area. (Photo 5)

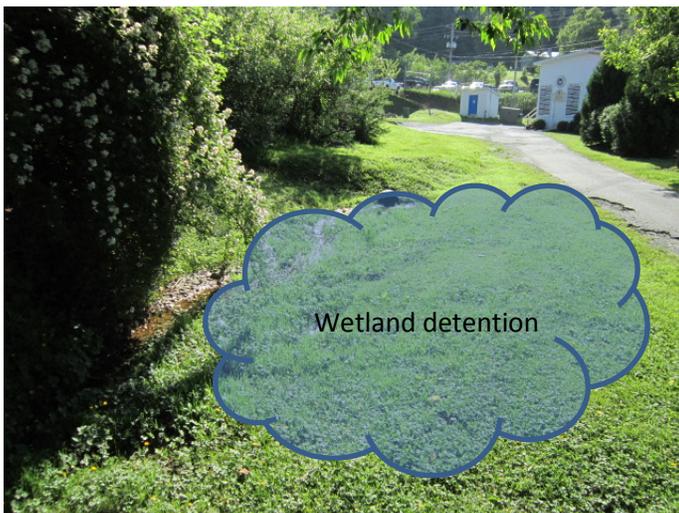
Drainage area = 26.27 acres
 Impervious surface = 4.92 acres

Affected stream = unnamed tributary to North Toe River
 Stream classification = WS-V, Tr



BMP solution

The area in Photo 1 has already been modified into a basin-like structure to capture and direct the stormwater. The site will be further modified into a functioning stormwater detention basin to store and treat the runoff from the higher culvert. This will reduce the runoff into the stream, gradually release it, or eliminate it totally. The two lower culverts will be connected in order to carry the streamflow and runoff to the area in Photo 3 without interfering with the proposed detention basin.



The current route of the stream/runoff parallels an open grassy area where the topography is conducive to the construction of connecting wetland detention cells appropriate to the character of the park. The stormwater will spread over a larger area with the capacity to store a significant amount of water, and pollutants will be filtered as the water slowly discharges from the cell.

Dry detention basin	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	120		88		32
COD	690		552		138
TSS	3,240		1,377		1,863
LEAD	4		2		2
COPPER	1		U		U
ZINC	4		3		1
TDS	3,390		U		U
TN	36		25		11
TKN	12		U		U
DP	2		U		U
TP	4		3		1
CADMIUM	0		U		U

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the larger creek. Sediment will be captured as well. Reduction or elimination of the stormwater volume entering the creek will mitigate flooding at the confluence and downstream; thus lessening erosion that often results from higher stream volumes.

Wetland detention	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	289		211		78
COD	1,865		1,492		373
TSS	4,046		1,719		2,326
LEAD	3		2		2
COPPER	1		U		U
ZINC	12		9		2
TDS	5,727		U		U
TN	81		57		24
TKN	42		U		U
DP	3		U		U
TP	11		8		3
CADMIUM	0		U		U

Cost estimate²

Dry detention basin	\$11,400	construction
	<u>570</u>	engineering
	\$11,970	
Wetland detention	\$14,268	construction
	<u>1,000</u>	engineering
	\$15,268	
TOTAL PROJECT COST	\$27,238	

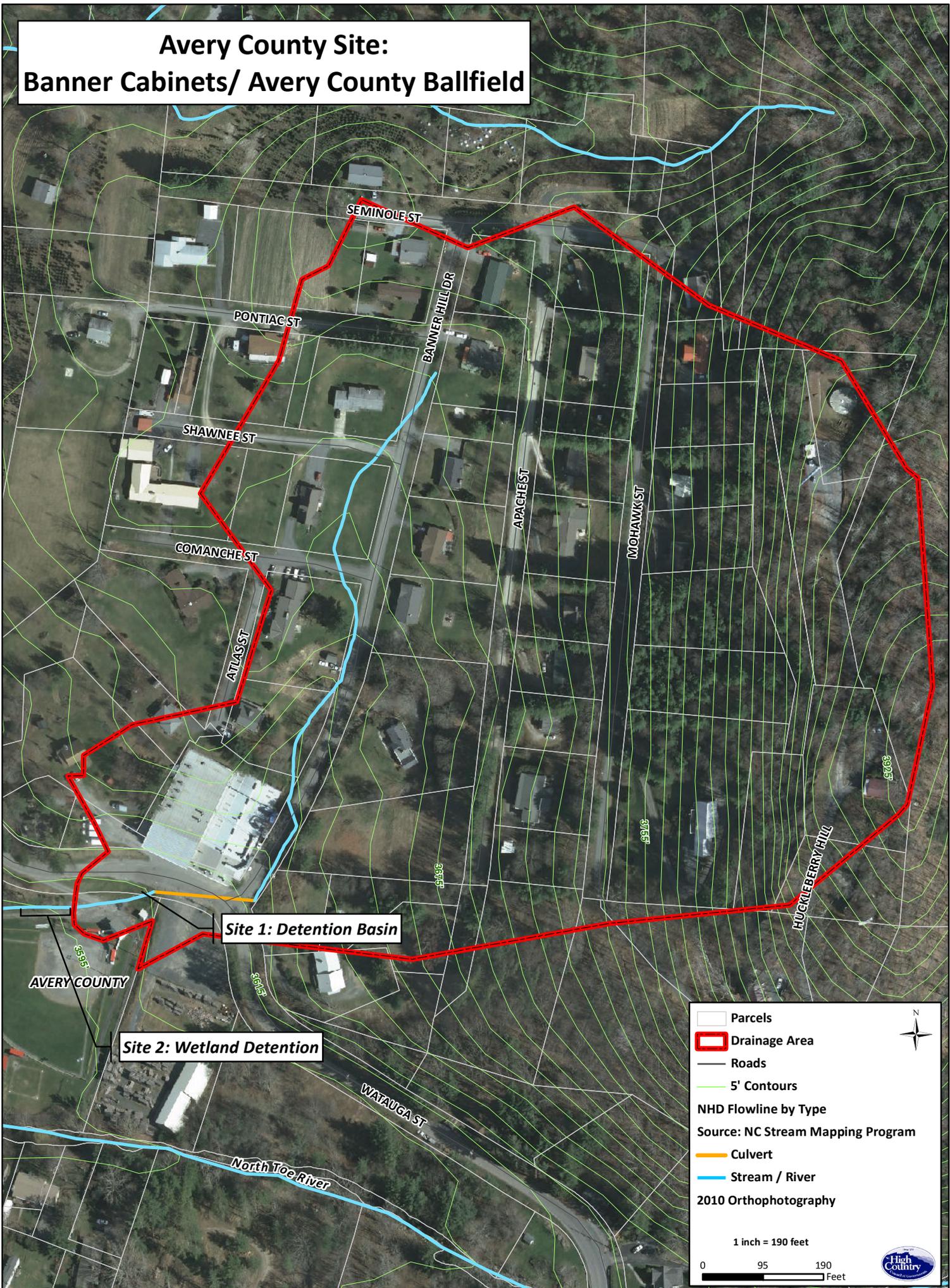
Funding

North Carolina Clean Water Management Trust Fund
 Soil & Water District Community Conservation Assistance Program
 North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Avery County Site: Banner Cabinets/ Avery County Ballfield



Legend

- Parcels
- Drainage Area
- Roads
- 5' Contours

NHD Flowline by Type
Source: NC Stream Mapping Program

- Culvert
- Stream / River

2010 Orthophotography

1 inch = 190 feet

0 95 190 Feet

Diagram and photograph of a typical detention basin:

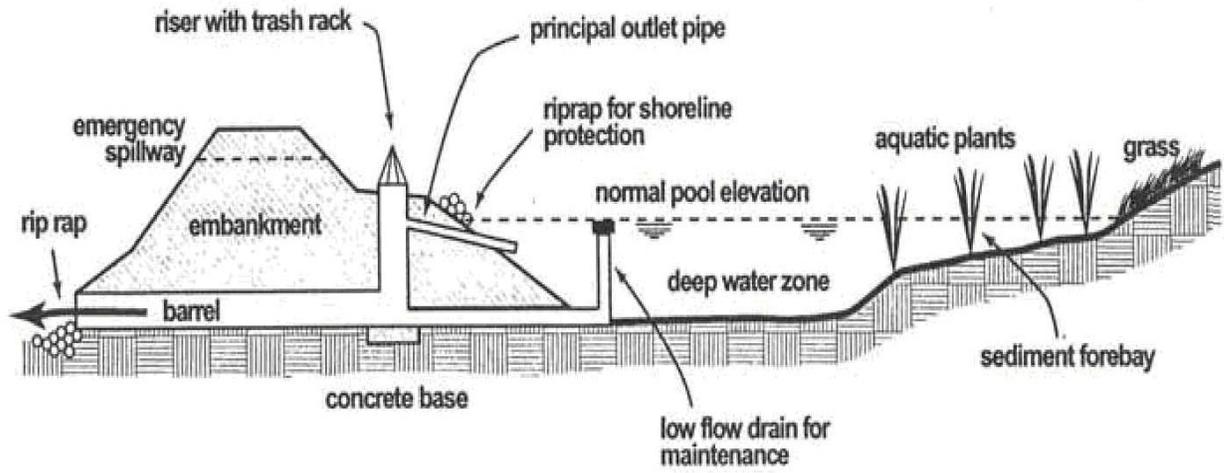
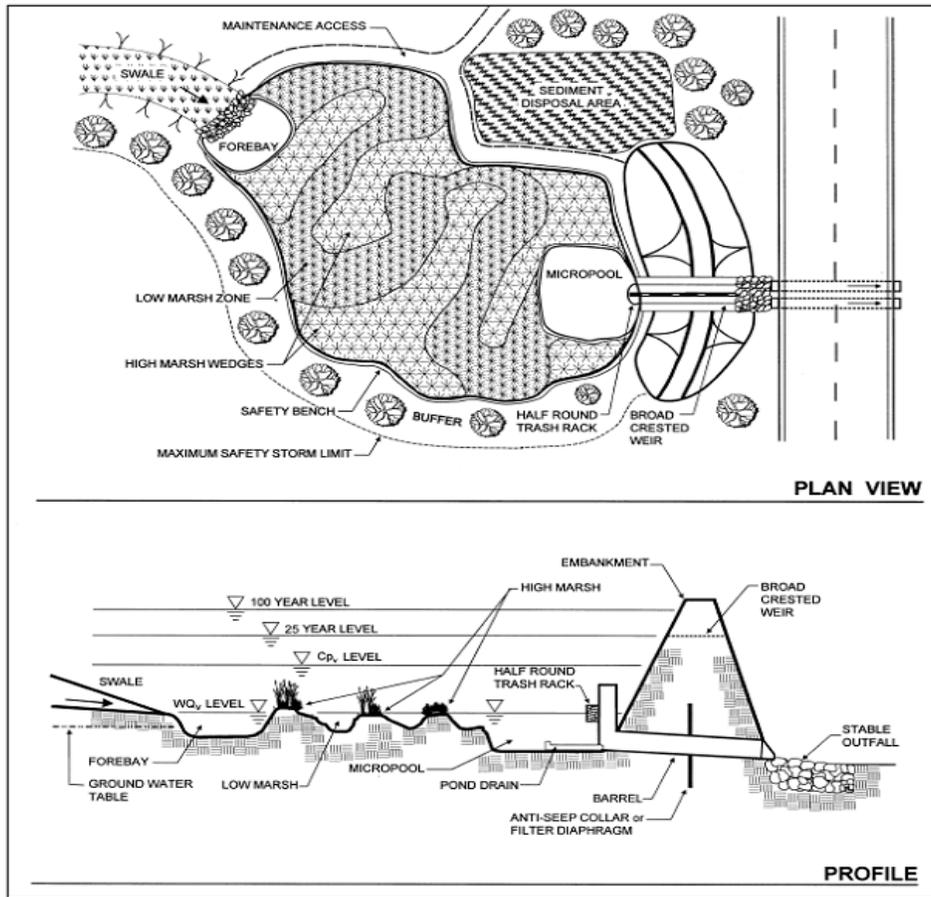


Diagram and photograph of a typical stormwater wetland:



High Country Council of Governments
Regional Stormwater Project
Town of Banner Elk



Tate Evans Park



Problem

Runoff from parking lots & drives is piped directly into Shawneehaw Creek. The creek was the focus of a major stream restoration effort and is a central feature of the Town park. The park is heavily used for active recreation, including the creek. The runoff from the paved surfaces introduces oils and grease, hydrocarbons, metals, and road salt to the stream, and can increase water temperature.

Drainage area = 0.92 acre
Impervious surface = 0.83 acre

Affected stream = Shawneehaw
Creek
Stream classification = C, Tr





BMP solution

Because the park is heavily used, an expansive BMP is not feasible. The lack of space can be mitigated by developing a linear BMP bordering the creek; in effect, a relatively shallow and narrow bioretention cell that will dissipate and absorb the flow over several linear feet as opposed to one that is conventionally deep and wide. The existing culvert (red arrow) will be sheared back a few feet to obtain an angle where runoff can flow into the proposed BMP (blue arrows). A flowsplitter will be installed to direct stormwater into the BMP while allowing extremely excessive flows to spill over into the former channel.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Reduction or elimination of the stormwater volume entering the creek will lessen erosion downstream associated with higher stream volumes.

Bioretention cell	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	33		12		21
COD	570		285		285
TSS	1,463		329		1,134
LEAD	2		1		1
COPPER	0		U		U
ZINC	2		1		1
TDS	3,922		U		U
TN	8		7		2
TKN	12		U		U
DP	0		U		U
TP	1		1		1
CADMIUM	0		U		U

Cost estimate²

Bioretention cell & flowsplitter	\$19,304
Design & engineering	<u>7,721</u>
	\$27,025

Funding

NC Clean Water Management Trust Fund

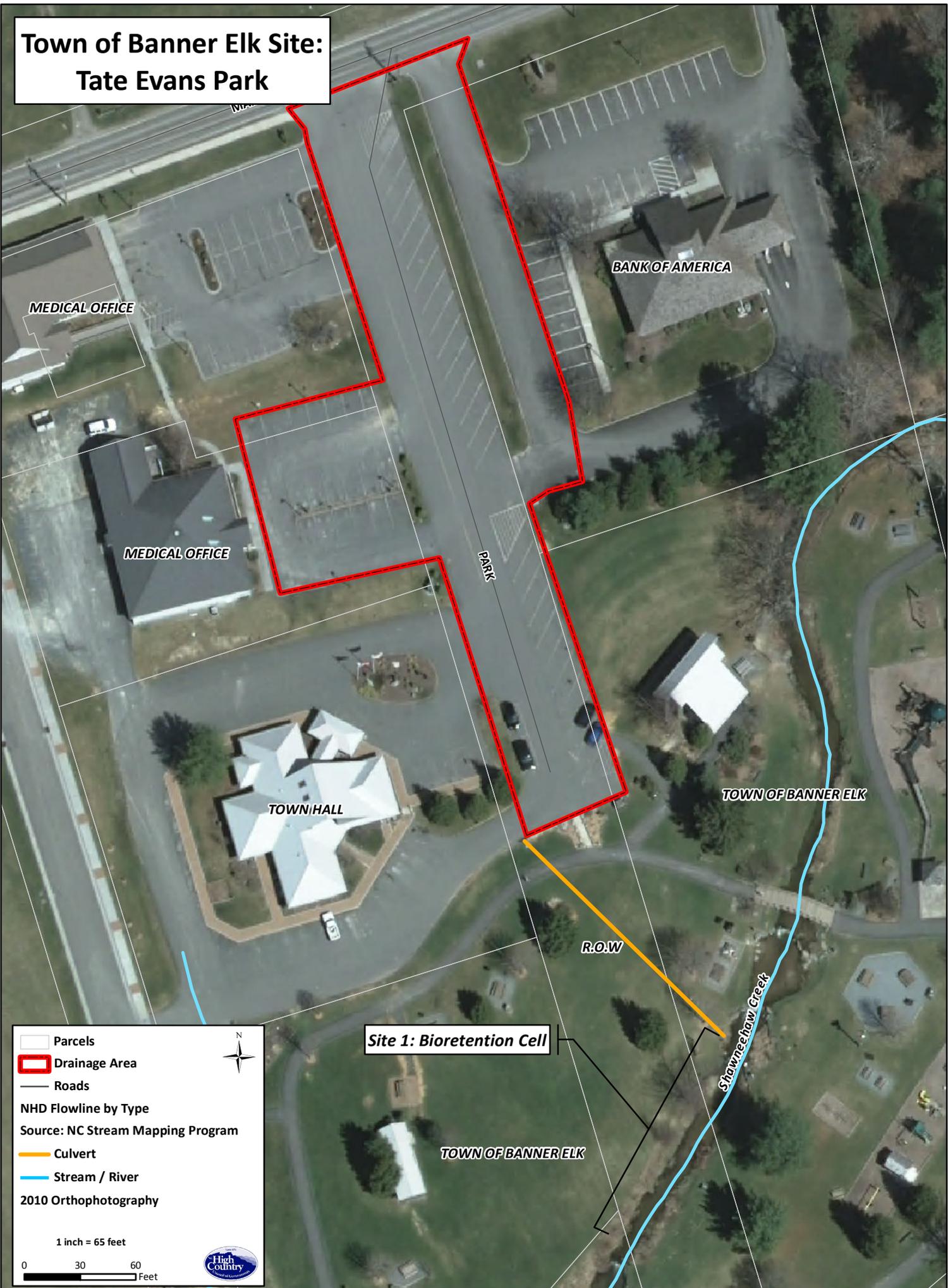
Soil & Water District Community Conservation Assistance Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Banner Elk Site: Tate Evans Park



MEDICAL OFFICE

MEDICAL OFFICE

BANK OF AMERICA

PARK

TOWN HALL

TOWN OF BANNER ELK

R.O.W

Shawnee/haw Creek

Site 1: Bioretention Cell

TOWN OF BANNER ELK

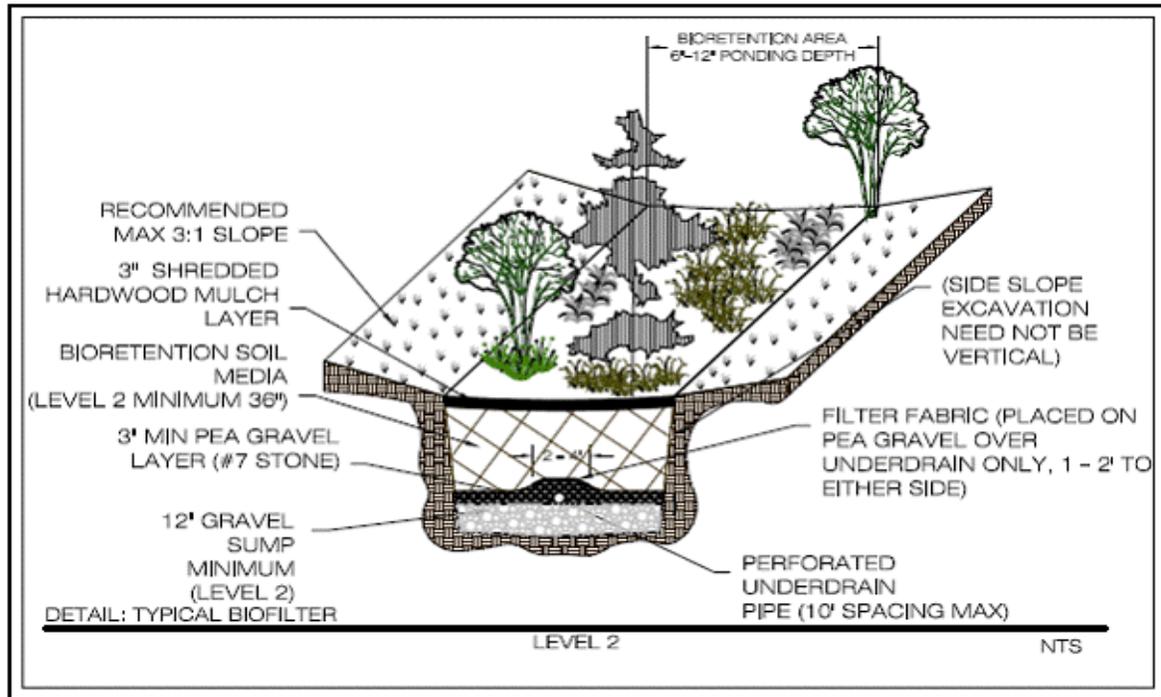
Legend:

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 65 feet

0 30 60 Feet

Diagram and photograph of a typical linear bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Crossnore



Town Center/Crossnore School



Problem

Runoff from one of the Crossnore School parking lot flows to a drain near Town Hall where it is directed into a stream.

Drainage area = 0.36 acre

Impervious surface = 0.15 acre; 41%

Affected stream = unnamed tributary to Crossnore Creek

Stream classification = C

BMP solution

The landscaped area will be expanded slightly (with no loss of parking space) and converted into a bioretention area. The site will be landscaped in a similar fashion, but the soil will be replaced with materials and a soil mix that will retain greater quantities of runoff. The addition of appropriate plants will enhance its filtering and retention capacity. A grade-level road drain will direct flow into the bioretention area.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots will be prevented from entering the stream. Elimination of the stormwater volume from the creek will help lessen erosion downstream that often results from higher stream volumes. The bioretention cell will also help filter road salts and reduce water temperature spikes.

Bioretention cell	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	11		2		9
COD	186		U		U
TSS	479		29		450
LEAD	1		U		U
COPPER	0		U		U
ZINC	1		U		U
TDS	1,283		U		U
TN	3		U		U
TKN	4		U		U
DP	0		U		U
TP	0		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$3,810
Design & engineering	<u>500</u>
	\$4,310

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Project Program
North Carolina Division of Water Resources Development Project Grant Program

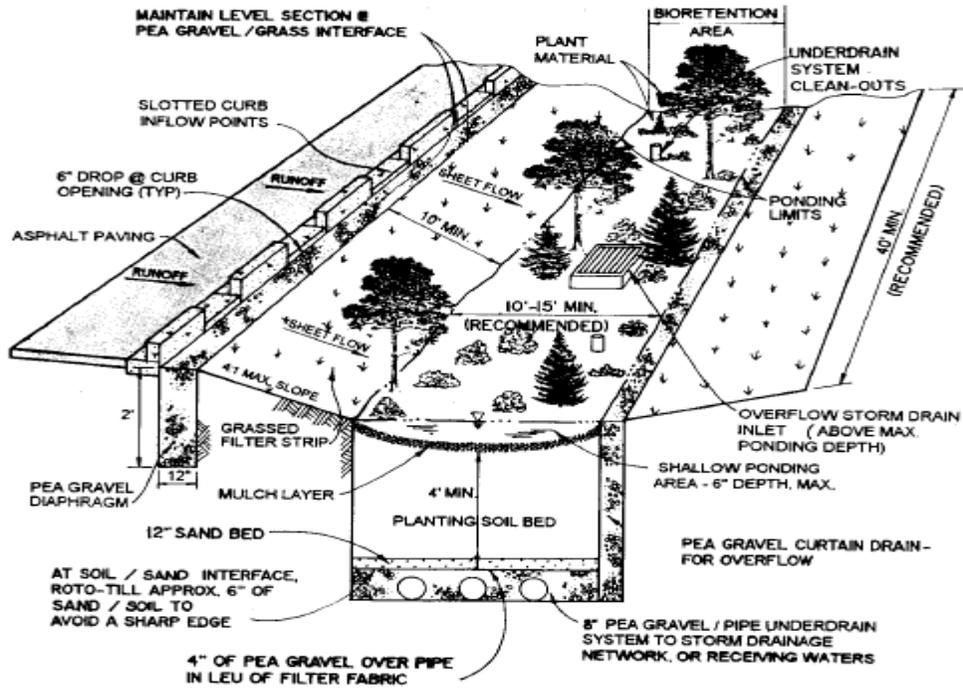
¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Crossnore: Town Center/Crossnore School



Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Newland



Old Public Road/North Toe River



Problem

Runoff from an area near the center of town concentrates in this headwater tributary stream to the Toe River, causing flooding problems and contributing pollutants to the streamflow. The drainage area consists of various urban land uses- residential, commercial, streets, and parking.

Drainage area = 76.14 acres

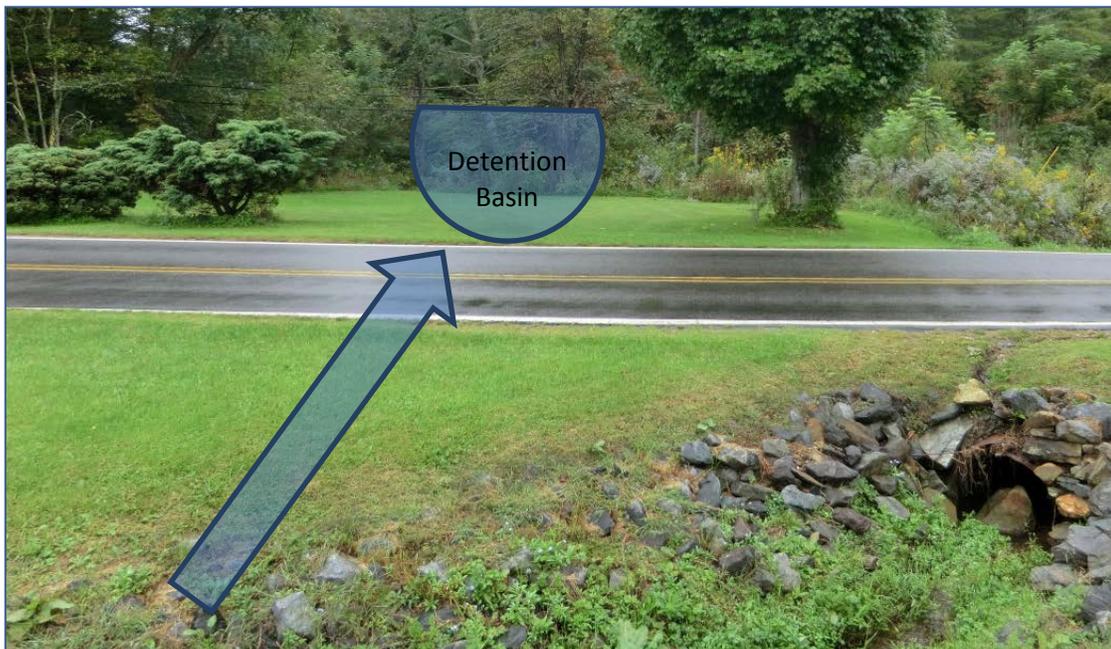
Impervious surface = 18.94 acres; 25%

Affected stream = unnamed tributary to North Toe River

Stream classification = WS-V, Tr

BMP solution

The area in the stream vicinity is heavily developed with little space for a BMP. A parcel of land across the road from the stream is owned by the Town and available for a BMP. A detention basin will be necessary to treat the high volume of stormwater in this drainage area. As indicated by the photo, the base flow of the stream is very low. A weir structure will be installed (blue arrow) on the stream to capture only the stormflow and divert it via a culvert across the road to the proposed detention basin. The detention basin will hold and filter the runoff to remove pollutants.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Reduction or elimination of the stormwater volume entering the creek will mitigate erosion associated with higher stream volumes.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	5,711		1,599		4,112
COD	39,593		U		U
TSS	79,186		11,086		68,100
LEAD	69		41		27
COPPER	11		11		11

Cost estimate²

Construction	\$41,668
Design & engineering	<u>\$ 2,083</u>
	\$43,751

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Project Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

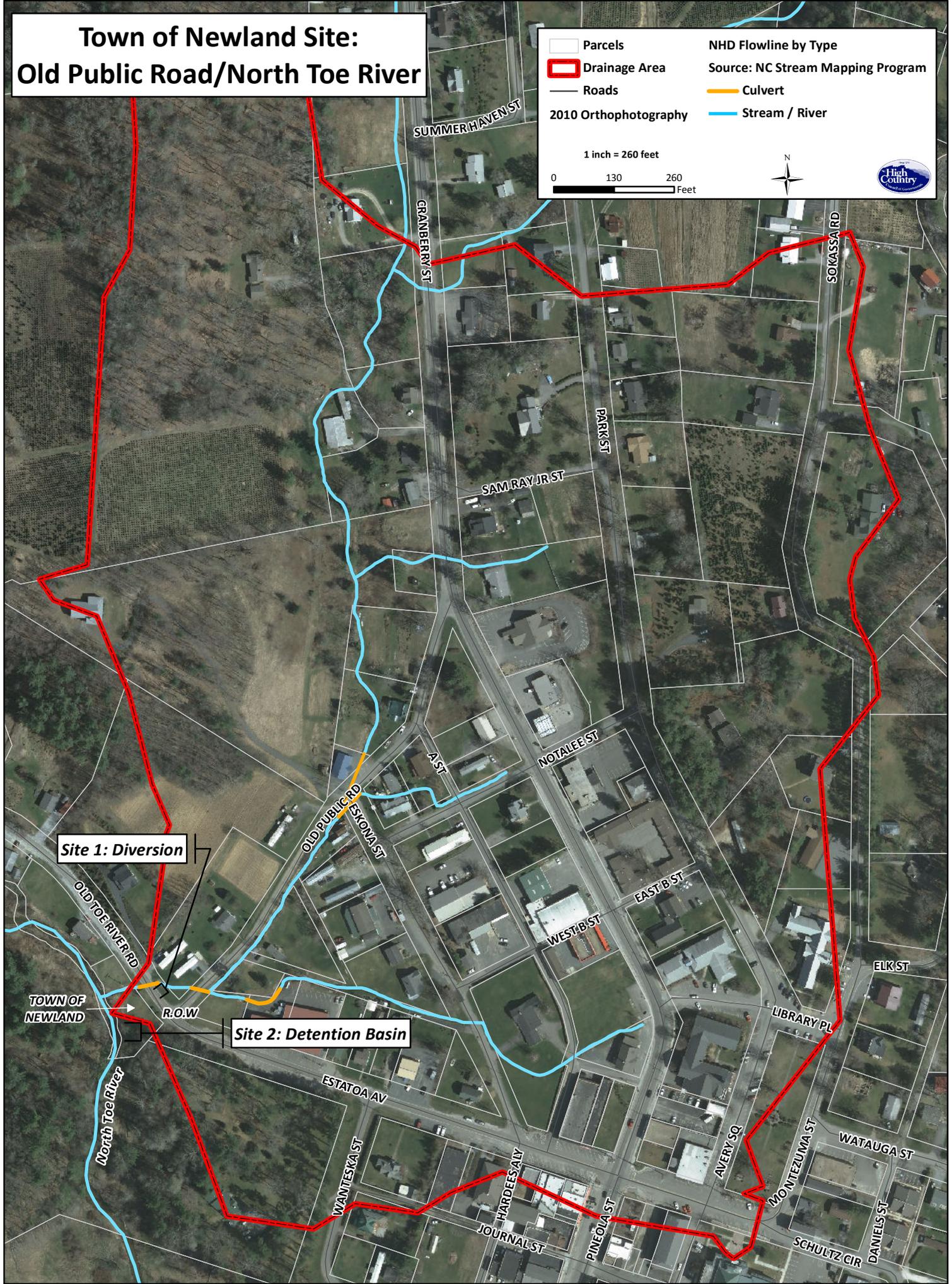
Town of Newland Site: Old Public Road/North Toe River

 Parcels	NHD Flowline by Type
 Drainage Area	Source: NC Stream Mapping Program
 Roads	 Culvert
2010 Orthophotography	 Stream / River

1 inch = 260 feet

0 130 260 Feet

 N

Site 1: Diversion

Site 2: Detention Basin

TOWN OF
NEWLAND

R.O.W

North Toe River

OLD TOE RIVER RD

OLD PUBLIC RD

SUMMER HAVEN ST

GRANBERRY ST

PARK ST

SAM RAY JR ST

SOKASSA RD

A ST

NOTALEE ST

EAST B ST

WEST B ST

ELK ST

LIBRARY PL

ESTATO AV

WATAUGA ST

WANTESKA ST

HARDEES ALY

AVERY SQ

MONTUZUMA ST

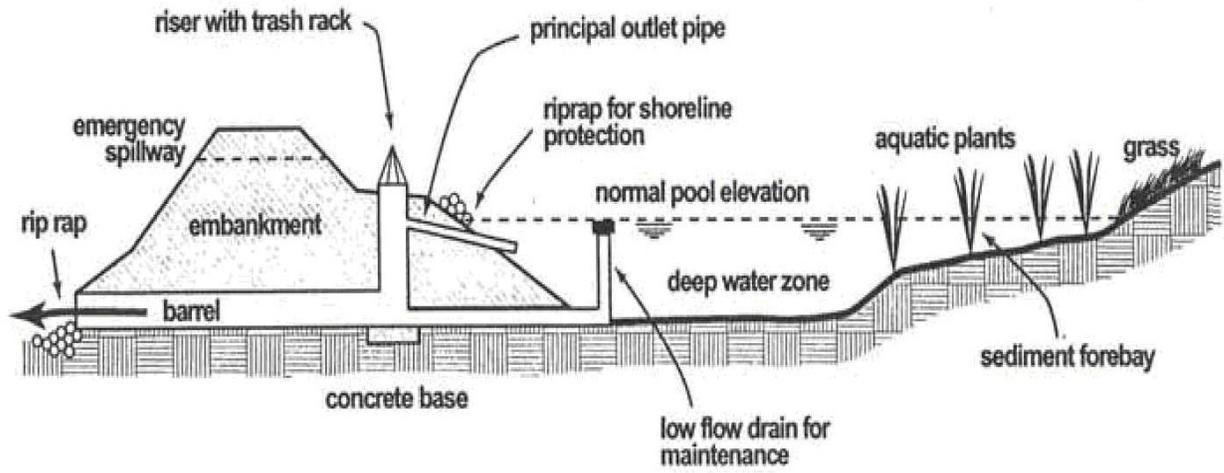
JOURNAL ST

PINEOLA ST

SCHULTZ CIR

DANIELS ST

Diagram and photograph of a typical detention basin:



High Country Council of Governments
Regional Stormwater Project
Village of Sugar Mountain



Craggy Pointe

Problem



Runoff from a street and parking area (photo above left) is causing severe erosion at several points downhill of the site (photos below and right).



Drainage area = 8.15 acres
Impervious surface = 1.41 acre; 17%

Affected stream = unnamed tributary to West Fork of Linville River
Stream classification = C Tr

BMP solution

A detention basin will be installed near the impervious area to capture, retain, and treat the runoff.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the receiving stream, located approximately 200 feet from the BMP site. Reducing the velocity of the runoff will also reduce erosion and sedimentation.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	42		7		35
COD	730		U		U
TSS	1,875		113		1,763
LEAD	2		U		U
COPPER	0		U		U
ZINC	3		U		U
TDS	5,027		U		U
TN	11		U		U
TKN	16		U		U
DP	0		U		U
TP	2		0		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$28,000
Design & engineering	<u>\$ 1,400</u>
	\$29,400

Funding

North Carolina Clean Water Management Trust Fund

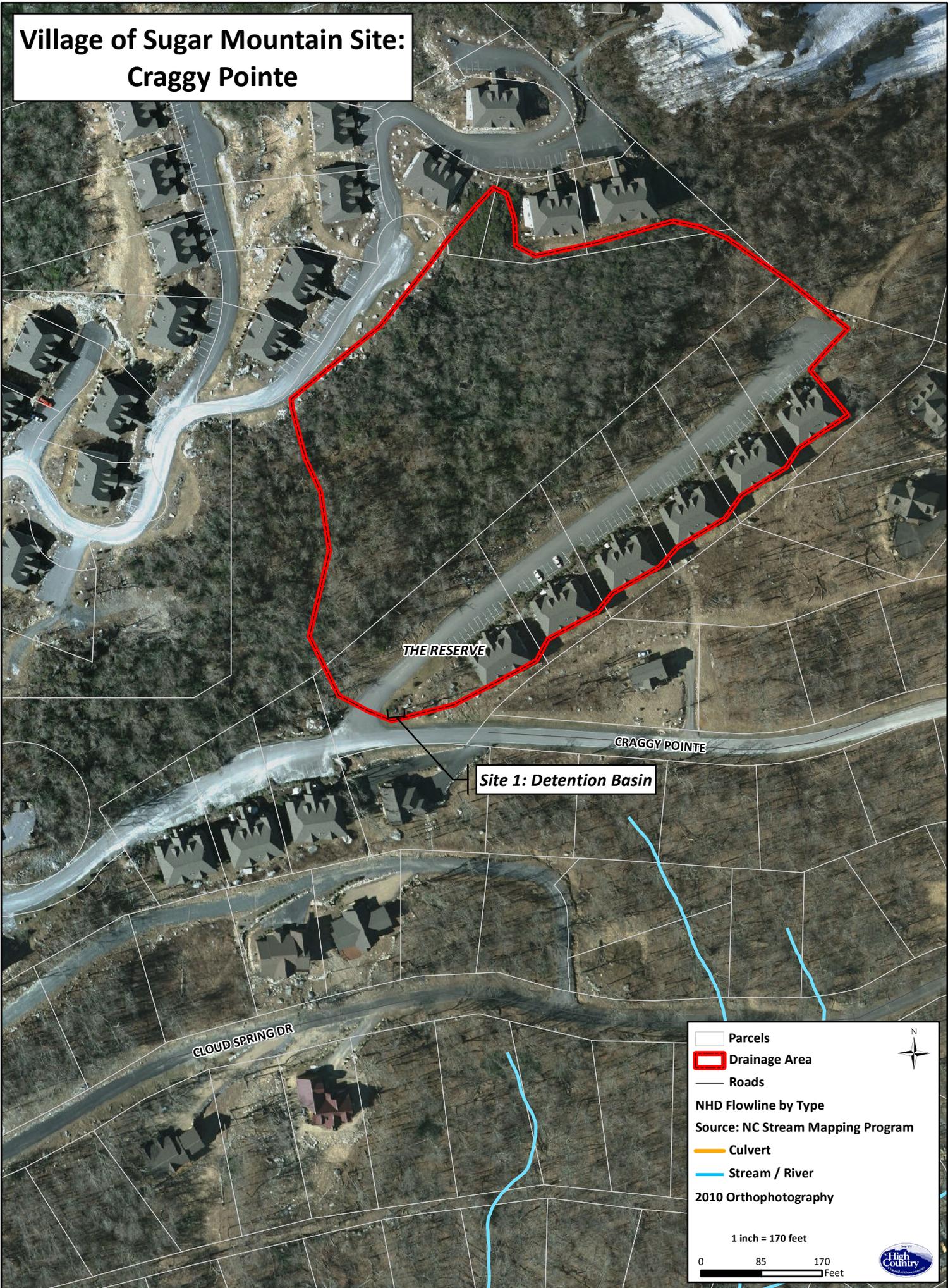
Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Village of Sugar Mountain Site: Craggy Pointe



THE RESERVE

Craggy Pointe

Site 1: Detention Basin

Cloud Spring Dr

Legend:

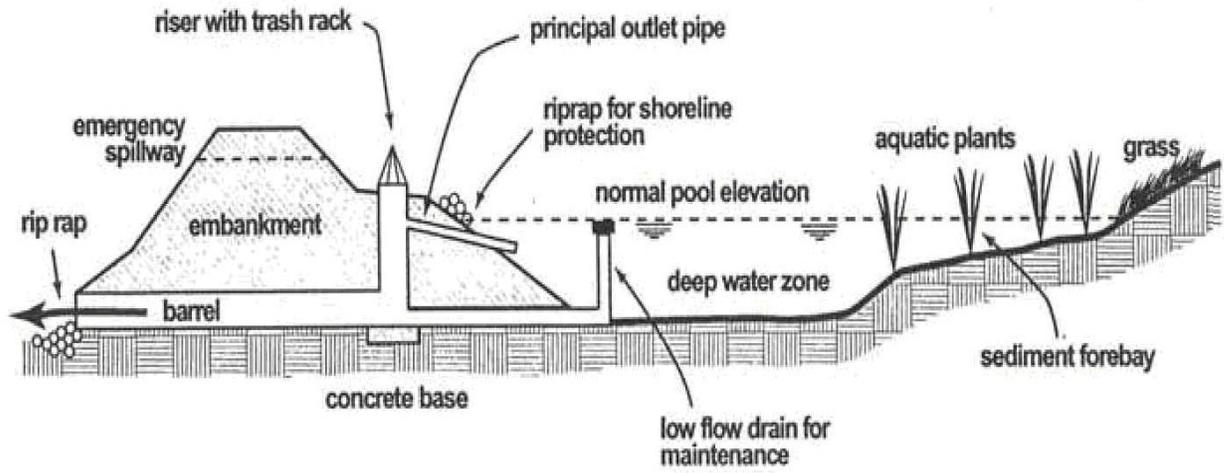
- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 170 feet

0 85 170 Feet

High Country

Diagram and photograph of a typical detention basin:



High Country Council of Governments
Regional Stormwater Project
Mitchell County



Mitchell County Senior Center



Problem

Several Mitchell County agency buildings occupy a site in the Ledger Community. Runoff from the site and the access road collects in a ditch in front of the Senior Center (brick building in photo) and the Board of Education building, and eventually reaches a creek. Erosion from the runoff is evident in several locations. Some stormflow from the road also reaches the ditch. The runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 15.64 acres

Impervious surface = 5.02 acres; 32%



Affected stream = unnamed tributary to Cranberry Branch

Stream classification = C, Tr

BMP solution

The drainage ditch's existing form will be used to convert it to a bioretention swale, replacing at least some of the soil with materials and a soil mix that will retain greater quantities of runoff. Space is available to widen it to increase its volume. The addition of appropriate plants will also enhance its filtering and retention capacity, and give this BMP a landscaped appearance that is needed at this highly visible location. A series of checkdams will slow the velocity to allow more infiltration.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. With erosion no longer an issue, sedimentation from the site will no longer enter the stream.

Vegetated swale	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	469		232		237
COD	8,102		4,861		3,241
TSS	20,801		5,616		15,185
LEAD	25		14		11
COPPER	5		U		U
ZINC	30		12		18
TDS	55,757		U		U
TN	120		72		48
TKN	172		U		U
DP	2		U		U
TP	17		9		8
CADMIUM	0		U		U

Cost estimate²

Construction	\$91,113
Design & engineering	<u>4,555</u>
	\$95,668

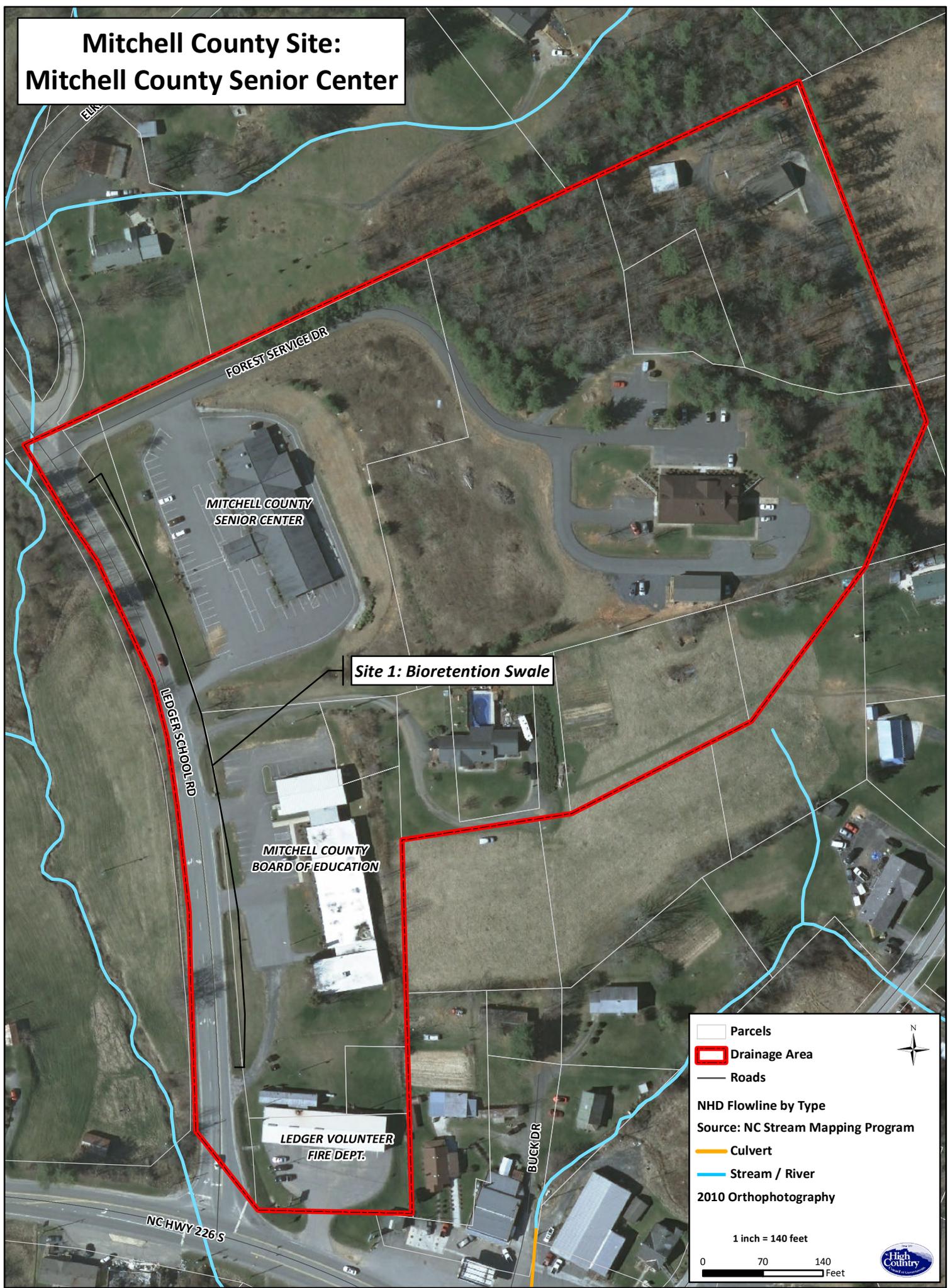
Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Mitchell County Site: Mitchell County Senior Center



Site 1: Bioretention Swale

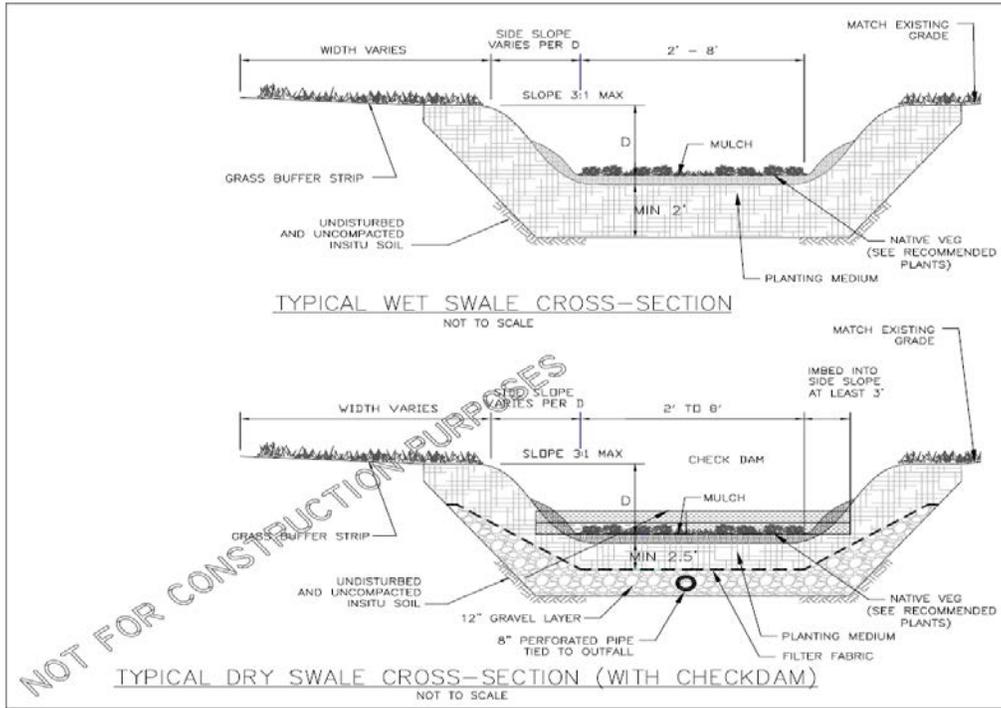
Legend:

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 140 feet

0 70 140 Feet

Diagram and photograph of a typical swale:



High Country Council of Governments
Regional Stormwater Project
Town of Bakersville



Creekwalk Trailhead



Problem

Runoff from the school campus and adjacent street is piped off-site and drains into the creek. The flattened vegetation indicates that the existing grassy area is insufficient to filter the high volume of stormwater. The runoff from the street, parking lots, and rooftops introduces oils and grease, hydrocarbons, metals, and road salt to the stream.



BMP solution

The site is adjacent to a town park that serves as a trailhead for a walking path. A linear bioretention cell with ornamental plantings will enhance the park environment while achieving the desired water quality objectives.

Drainage area = 6.53 acres
Impervious surface = 2.45 acres; 37%
Affected stream = White Oak Creek
Stream classification = C, Tr

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. Temperature fluctuations caused by this site's runoff will be eliminated.

bioretention	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	196		72		123
COD	3,383		1,691		1,691
TSS	8,685		1,954		6,731
LEAD	10		4		7
COPPER	2		U		U
ZINC	12		8		4
TDS	23,279		U		U
TN	50		40		10
TKN	72		U		U
DP	1		U		U
TP	7		4		3
CADMIUM	0		U		U

Cost estimates²

Construction	\$11,938
Design & engineering	<u>1,193</u>
	\$13,131

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Bakersville Site: Creekwalk Trailhead

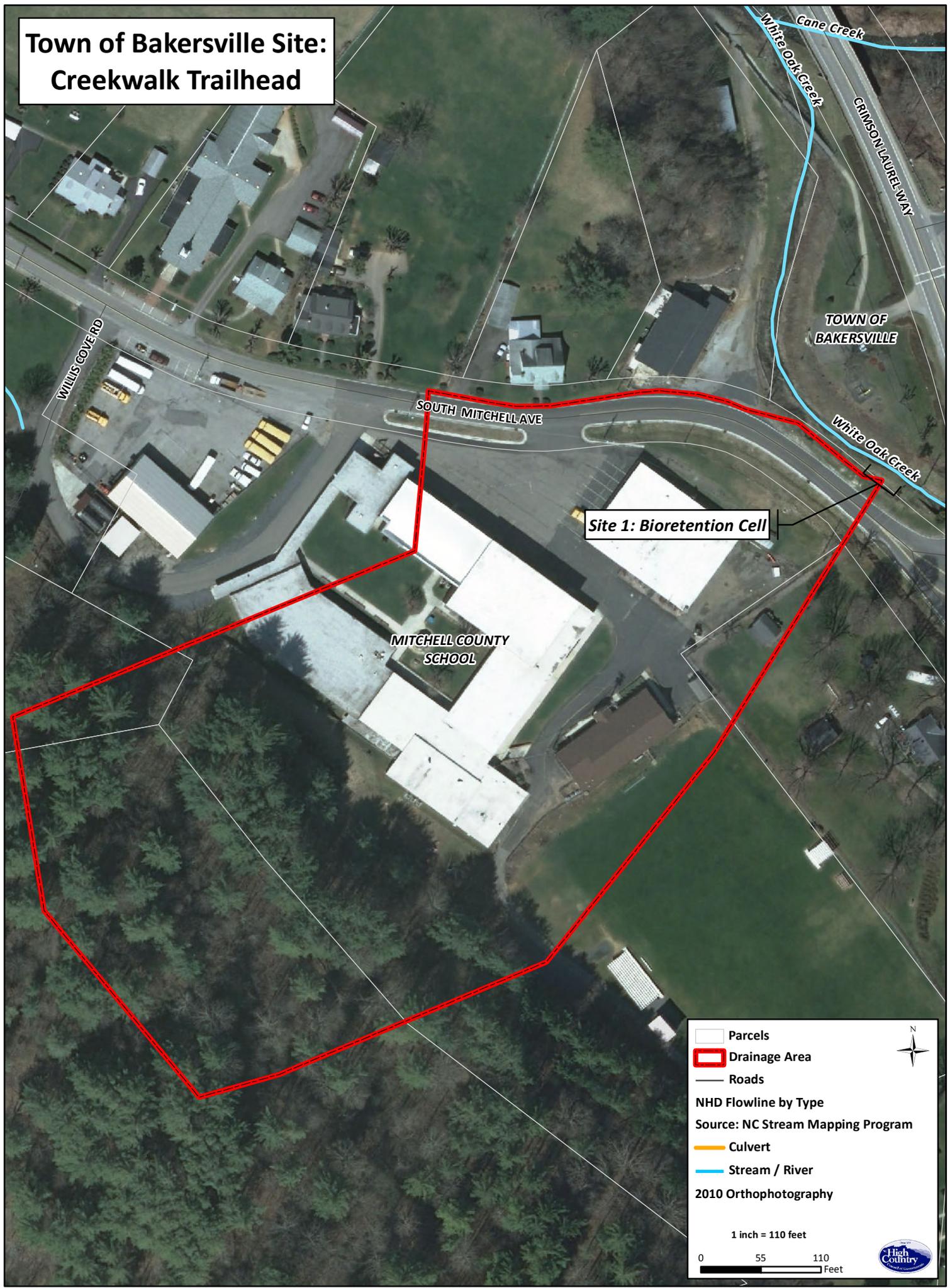
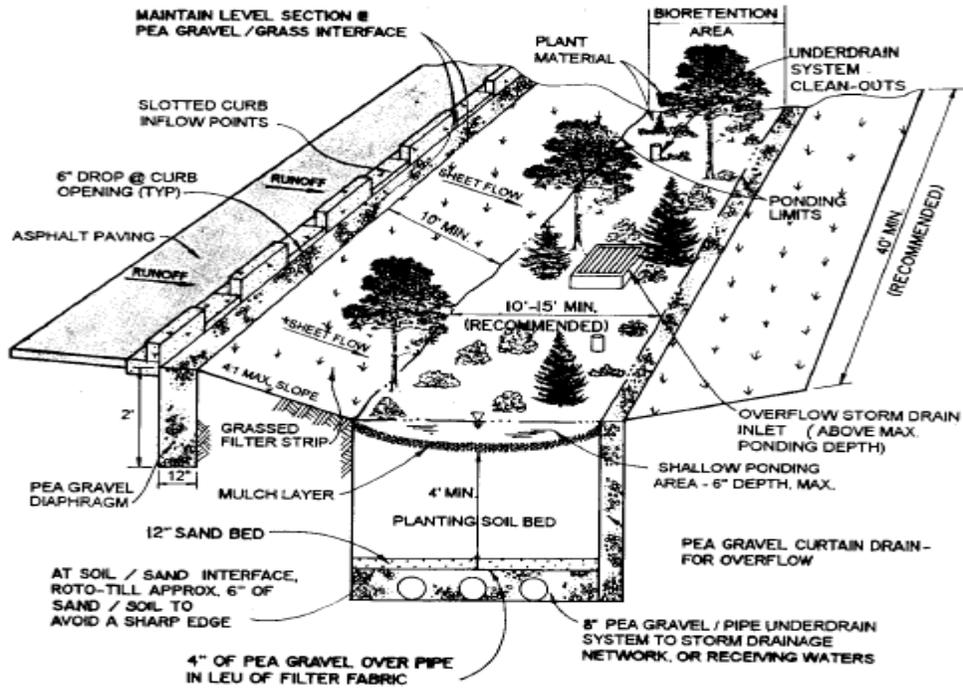


Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Bakersville



Hemlock Drive Parking Lot

Problem

Runoff from the parking lot flows off the lot through a 15' grass area and into the creek. While the grass may capture much of the sheetflow, flattened vegetation shows that some stormwater concentrates at specific locations and flows unimpeded into the stream. The runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 0.5 acre
Impervious surface = 0.46 acre; 92%

Affected stream = Honeycutt Branch
Stream classification = C, Tr



BMP solution

Because the Town desires to maintain a trimmed border, the grassy area will be enhanced with bioretention features that will avoid an overgrown, messy appearance. A portion of the soil will be replaced with appropriate bioretention media. Landscape features, such as ornamental bunch grass, will be planted to capture and filter runoff, with thicker plantings at spots where the runoff tends to concentrate.

Water quality benefits

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. Temperature fluctuations caused by this site's runoff will be eliminated.

Vegetated filter strip ¹	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	74		36		37
COD	1,269		761		508
TSS	3,259		880		2,379
LEAD	4		2		2
COPPER	1		U		U
ZINC	5		2		3
TDS	8,734		U		U
TN	19		11		8
TKN	27		U		U
DP	0		U		U
TP	3		1		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$4,900
Design & engineering	<u>500</u>
	\$5,400

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Program

North Carolina Division of Water Resources Development Project Grant Program

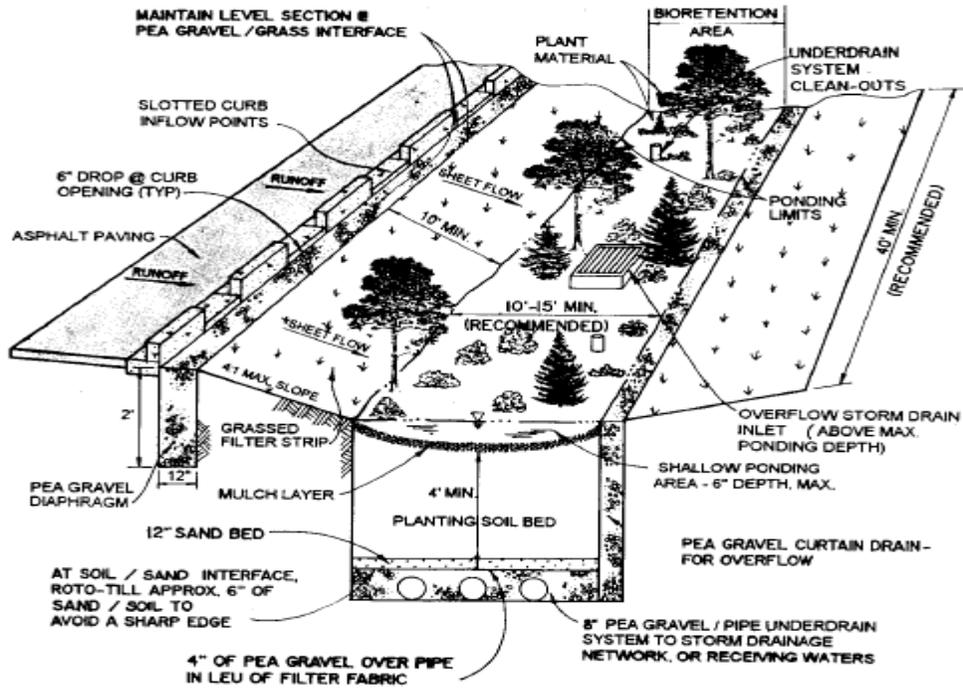
¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Bakersville Site: Hemlock Drive Parking Lot



Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Spruce Pine



Brad Ragan Park

Problem

The park contains several areas where runoff from parking lots & roads is diverted directly into a stream. The runoff introduces oils and grease, hydrocarbons, metals, and road salt to the receiving stream.

Drainage area = 1.69 acres

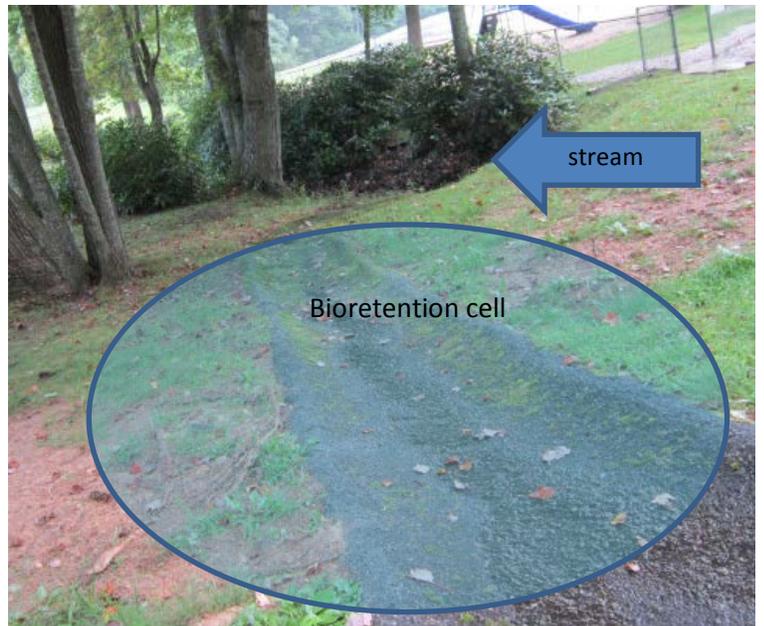
Impervious surfaces = 1.06 acres; 63%

Affected stream = unnamed tributary to English Creek

Stream classification = C, Tr

BMP solution

Four bioretention cells will be installed at separate locations to retain runoff & prevent stormwater from entering streams. Bioretention is the preferred BMP because the park environment will require a treatment that will blend well with the landscape and be unobtrusive to recreation activities. The park is heavily used by school groups and therefore the stormwater treatments will also serve an educational purpose with interpretive signage.





Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that is often associated with higher stream volumes. Temperature fluctuations caused by this site's runoff will be eliminated.

Bioretention cells	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	51		25		26
COD	875		525		350
TSS	2,248		607		1,641
LEAD	3		1		1
COPPER	1		U		U
ZINC	3		1		2
TDS	6,025		U		U
TN	13		8		5
TKN	19		U		U
DP	0		U		U
TP	2		1		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$26,924
Design & engineering	2,600
	\$29,524

Funding

North Carolina Clean Water Management Trust Fund
 Soil & Water District Community Conservation Assistance Program
 North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

**Town of Spruce Pine Site:
Brad Ragan Park #1**

Site 1: Bioretention Cell

Site 2: Bioretention Cell

TOWN OF
SPRUCE PINE

TOWN OF
SPRUCE PINE

LAUREL CREEK CT

Legend:

- Parcels (white outline)
- Drainage Area (red outline)
- Roads (black line)
- NHD Flowline by Type (Source: NC Stream Mapping Program)
- Culvert (orange line)
- Stream / River (blue line)
- 2010 Orthophotography

1 inch = 50 feet

0 25 50 Feet

**Town of Spruce Pine Site:
Brad Ragan Park #2**



TOWN OF
SPRUCE PINE

LAUREL CREEK CT

Site 1: Bioretention Cell

Site 2: Bioretention Cell

Legend:

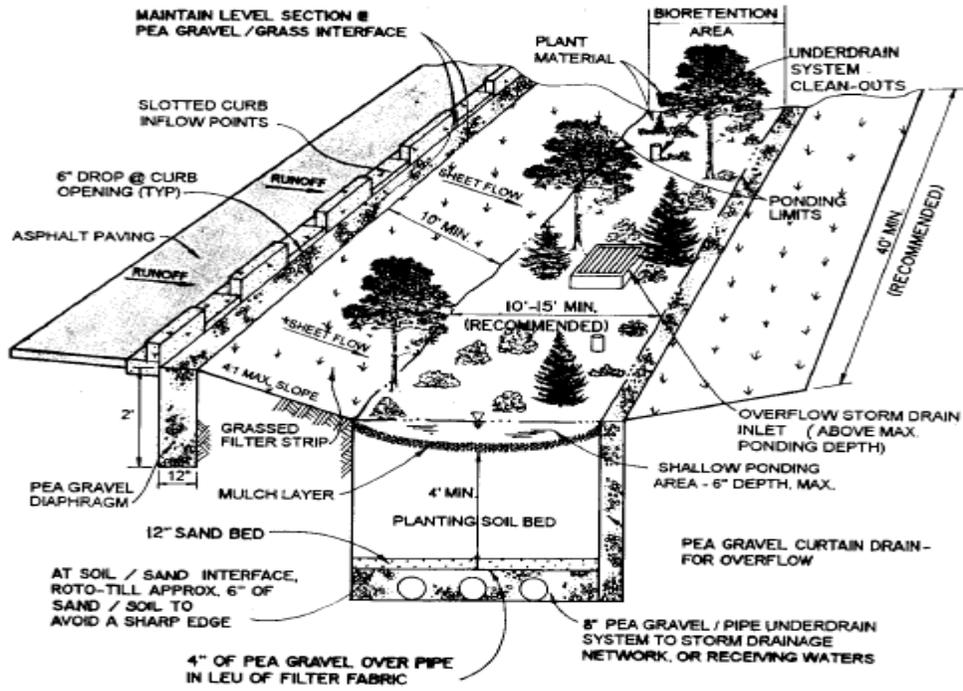
- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 42 feet

0 20 40 Feet

High Country

Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Watauga County



**Cooperative Extension
Parking Lot**

Problem

Runoff from two connected parking lots serving County agencies in downtown Boone drains directly into Boone Creek. Boone Creek flows through downtown Boone and Appalachian State University (ASU) campus. It is culverted for most of its length in downtown, and is impacted by point and non-point source pollution sources. Boone Creek is the focus of many restoration projects.



On the Cooperative Extension parking lot, runoff is directed to a single catch basin over the culverted creek.

Drainage area = 0.35 acre

Impervious surface = 0.35 acre; 100%

Affected stream = Boone Creek

Stream classification = C, Tr +

BMP solution

Stormwater will be directed away from the catch basin via a grade-level drain (blue lines in photo above) to a point where it will flow to a bioretention cell at the back of the lot (red arrow in photo above).

EMS Parking Lot

Problem



On the EMS parking lot, runoff is channeled directly into Boone Creek through a concrete spillway (photo above right).

Drainage area = 0.34 acre

Impervious surface = 0.34 acre; 100%

Affected stream = Boone Creek

Stream classification = C, Tr +

BMP solution

A bioretention cell will be constructed at the point where the runoff collects at the spillway.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will help lessen erosion downstream that often results from higher stream volumes. Temperature fluctuations associated with runoff from this site will be reduced. Efforts are being made to restore Boone Creek to a more natural condition with diverse objectives related to water quality, flood mitigation, and aesthetics. This BMP will help achieve the goals of that overall project.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	21		4		17
COD	357		U		U
TSS	918		55		863
LEAD	1		U		U
COPPER	0		U		U
ZINC	1		U		U
TDS	2,460		U		U
TN	5		U		U
TKN	8		U		U
DP	0		U		U
TP	1		0		1
CADMIUM	0		U		U

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

Cost estimate²

Construction	\$17,526
Design & engineering	<u>\$ 876</u>
	\$18,402

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Watauga County Site: Cooperative Extension Parking Lot



Parcels
 Drainage Area
 Roads

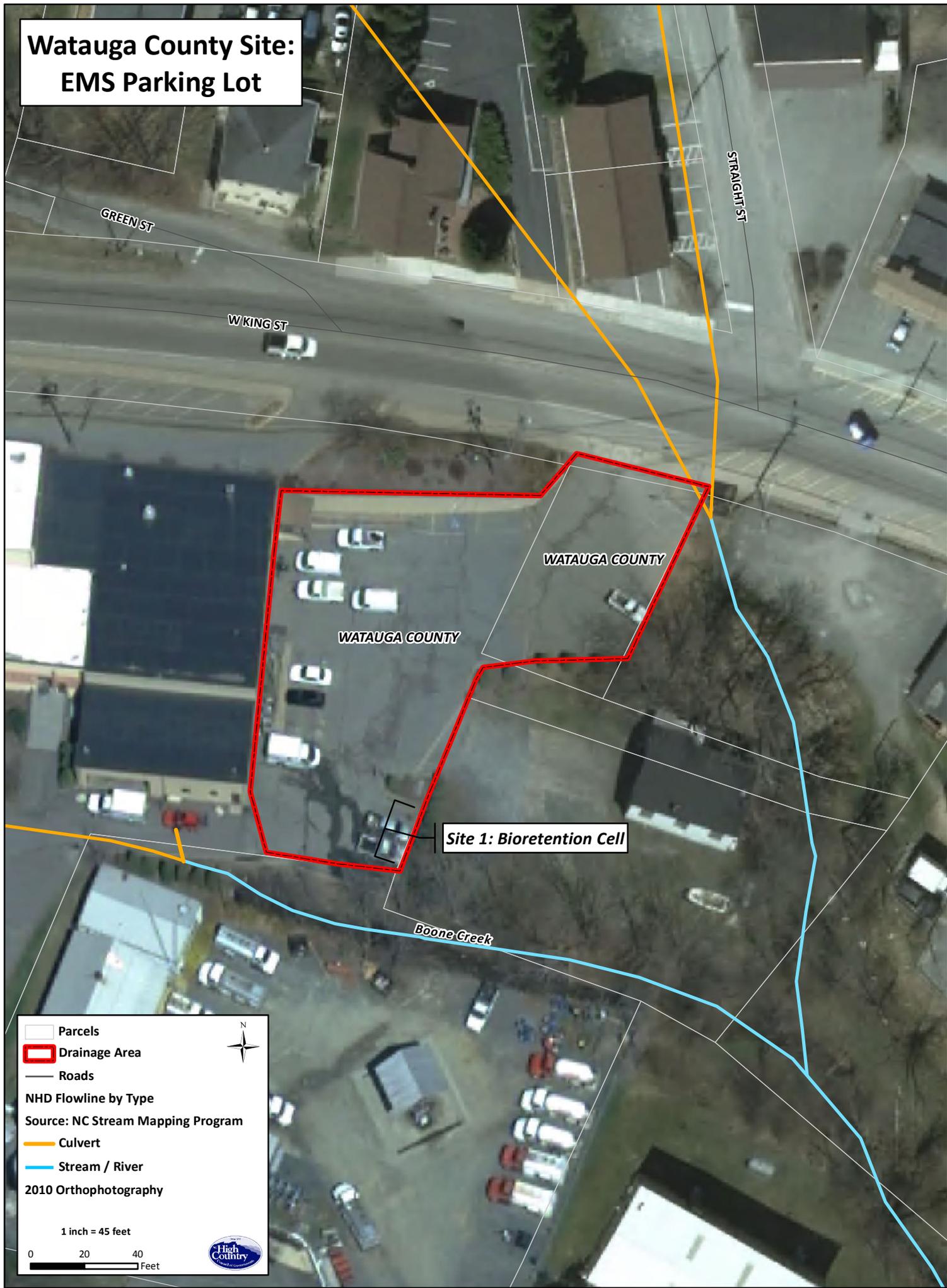
NHD Flowline by Type
Source: NC Stream Mapping Program
 Culvert
 Stream / River

2010 Orthophotography

1 inch = 45 feet

0 20 40 Feet

Watauga County Site: EMS Parking Lot



GREEN ST

W KING ST

STRAIGHT ST

WATAUGA COUNTY

WATAUGA COUNTY

Site 1: Bioretention Cell

Boone Creek

Parcels

Drainage Area

Roads

NHD Flowline by Type

Source: NC Stream Mapping Program

Culvert

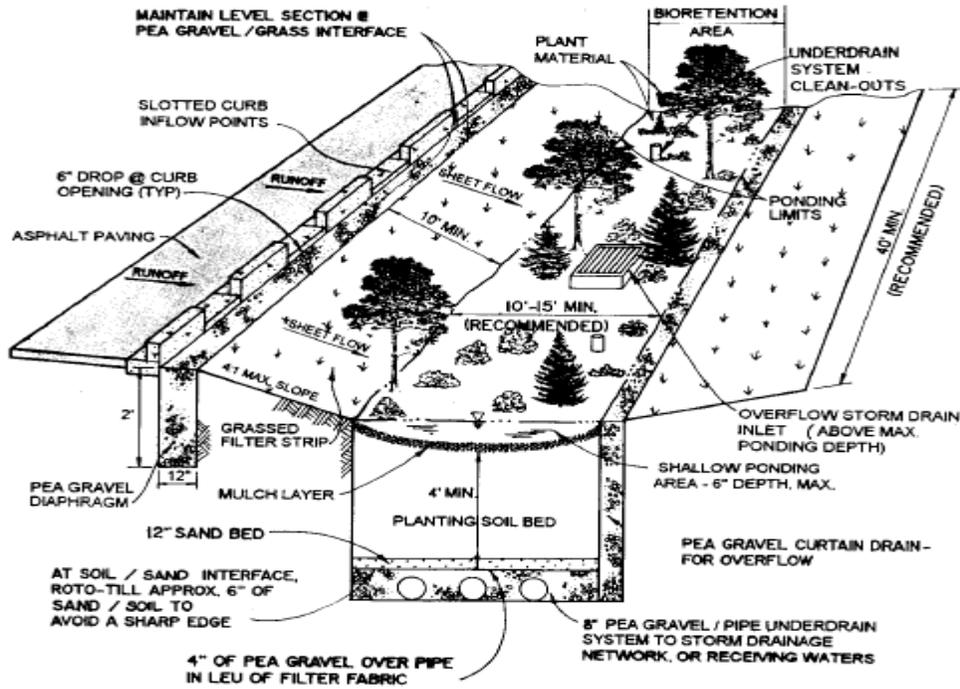
Stream / River

2010 Orthophotography

1 inch = 45 feet

0 20 40 Feet

Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Blowing Rock



Maple Street Parking Lot



Problem

Runoff from the parking lot flows offsite to neighboring properties. The runoff contributes to stormwater flow from other locations in town, channeling approximately 400 feet east of the site and draining into a perennial stream approximately 1,200 feet from the site. The entire area is urbanized, consisting of commercial development and residential development on small lots. The runoff from the paved areas introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 0.84 acre
Impervious area = 0.77 92%

Affected stream = unnamed tributary to Middle Fork, South Fork New River
Stream classification = WS-IV +



BMP solution

The existing vegetated border area, located downslope of the parking lot, is ideal for conversion to a linear bioretention cell to capture and filter the runoff. Bioretention will enhance the appearance of the lot, which is centrally located, highly visible, and heavily used by visitors. A grade-level drain may be necessary to direct flow to the border area in the background.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots will be prevented from entering the stream. Elimination of the stormwater volume from the creek will help lessen erosion downstream that often results from higher stream volumes.

Bioretention cells	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	25		U		U
COD	435		152		283
TSS	1,117		279		838
LEAD	1		0		1
COPPER	0		U		U
ZINC	2		1		1
TDS	2,995		U		U
TN	6		3		4
TKN	9		U		U
DP	0		U		U
TP	1		0		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$19,558
Design & engineering	<u>1,000</u>
	\$20,558

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Blowing Rock Site: Maple Street Parking Lot

Site 1: Bioretention Cell

Site 2: Bioretention Cell

FIRST CITIZENS BANK & TRUST

TOWN OF BLOWING ROCK

MAPLEST

MAPLEST

SUNSET DR

Legend:

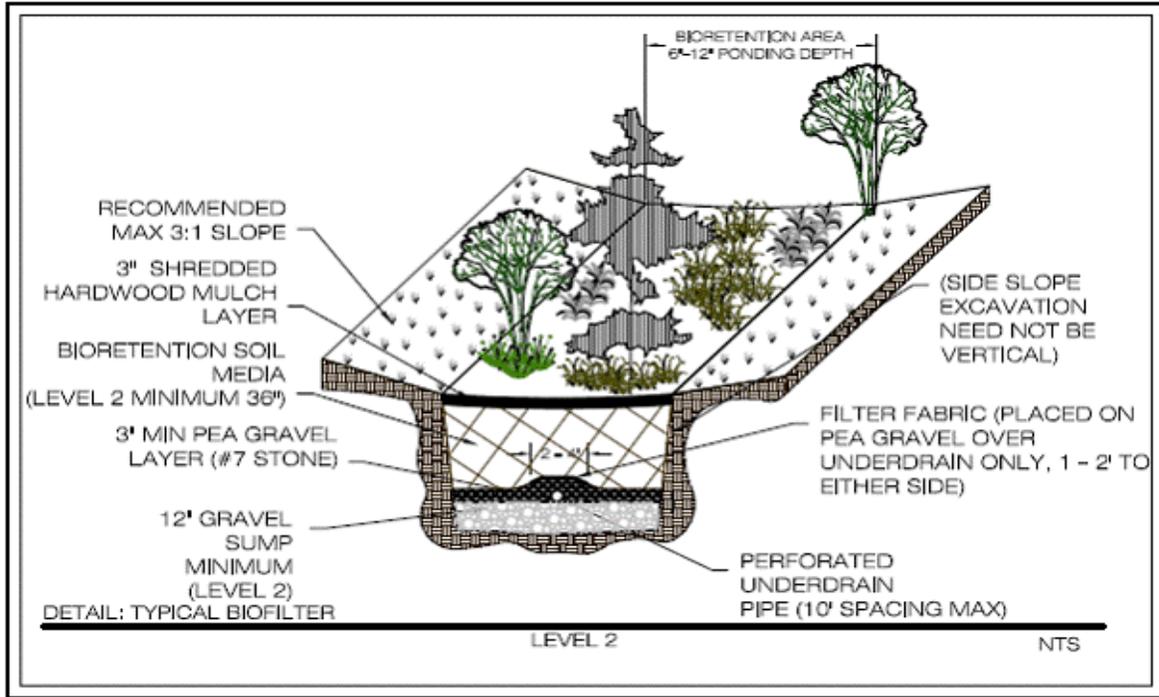
- Parcels (white outline)
- Drainage Area (red dashed outline)
- Roads (black line)
- NHD Flowline by Type (Source: NC Stream Mapping Program)
- Culvert (yellow line)
- Stream / River (blue line)
- 2010 Orthophotography

1 inch = 50 feet

0 25 50 Feet



Diagram and photograph of a typical linear bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Boone



Appalachian State University, Duncan Hall



Problem

A stormdrain (below) flows to this concrete channel (upper left photo) which runs directly into the stream. The channel also captures overland flow from the surrounding area (right photos; arrow shows location of channel). While the existing grassy areas provide some filtration for this sheetflow, observation during a rain event revealed that it is not sufficient for the high volume and rapid velocity of the runoff. As flow in the receiving stream increases during storm events, stormwater backs up into the concrete channel and surrounding area.

Drainage area = 0.31 acre
Impervious surface = 0.17 acre

Affected stream = Boone Creek
Stream Classification = C, Tr +



BMP solution

The site has an obvious swale form and drainage pattern that can easily be converted into a linear bioretention area with minimal disturbance. After removing the concrete channel, the area will be excavated to have sufficient depth to hold and filter stormwater before it reaches the stream. Vegetation in addition to grass will further enhance these BMP outcomes.

Water quality benefits¹

In the winter, the area's sidewalks, drives, and parking lots are heavily coated with salt and/or ice-melt chemicals. By capturing and treating the runoff, salt, heavy metals, and other pollutants associated with roadways will be prevented from entering the stream. Elimination of the stormwater volume from the creek will help lessen erosion downstream that often results from higher stream volumes. Efforts are being made to restore Boone Creek to a more natural condition with diverse objectives related to water quality, water temperature fluctuations, flood mitigation, and aesthetics. This BMP will help achieve the goals of that overall project.

Vegetated swale	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	9		5		5
COD	161		96		64
TSS	412		111		301
LEAD	0		0		0
COPPER	0		U		U
ZINC	1		0		0
TDS	1,105		U		U
TN	2		1		1
TKN	3		U		U
DP	0		U		U
TP	0		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$3,085
Design & engineering	\$ 500
	\$3,585

Funding

NC Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Boone Site: Appalachian State University, Duncan Hall



Legend:

- Parcels
- Drainage Area
- Roads

NHD Flowline by Type

Source: NC Stream Mapping Program

- Culvert
- Stream / River

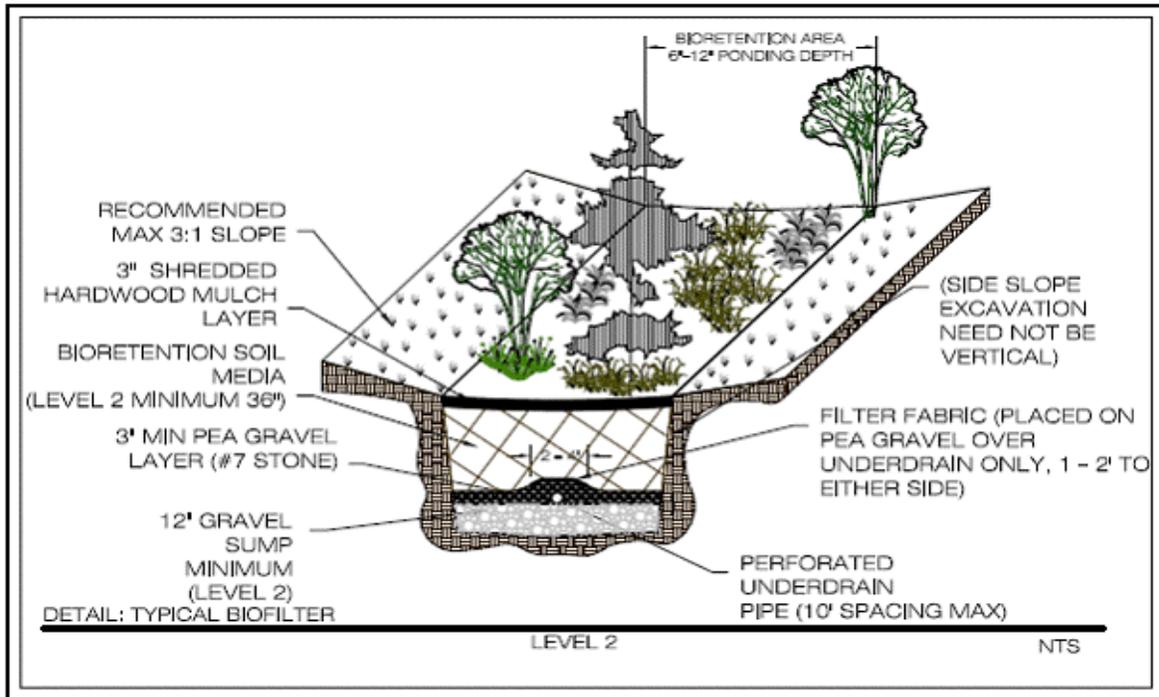
2010 Orthophotography

1 inch = 60 feet

0 30 60 Feet



Diagram and photograph of a typical linear bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Wilkes County



Wilkes County Senior Center

Problem



Runoff from the building and parking lots is directed into a ditch, causing erosion and thereby introducing sediment into Long Creek in addition to pollutants from the stormwater.



Drainage area = 2.33 acres
Impervious surface = 1.76 acres; 76%

Affected stream = Long Creek
Stream classification = C

BMP solution

The ditch will be converted to a bioretention swale, replacing at least some of the soil with materials and a soil mix that will retain greater quantities of runoff. The addition of appropriate plants will increase its filtering and retention capacity while also enhancing the appearance of this public site. A series of checkdams will slow the velocity to allow more infiltration.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. The bioretention swale will also reduce erosion and sedimentation within the ditch. Reduction of stormwater velocity will help lessen erosion downstream that often results from higher stream volumes.

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	70		35		35
COD	1,207		724		483
TSS	3,099		837		2,262
LEAD	4		2		2
COPPER	1		U		U
ZINC	4		2		3
TDS	8,306		U		U
TN	18		11		7
TKN	26		U		U
DP	0		U		U
TP	3		1		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$19,184
Design & engineering	<u>\$ 1,083</u>
	\$20,267

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Wilkes County Site: Wilkes County Senior Center

Site 1: Bioretention Swale

Site 2: Bioretention Swale

Site 3: Bioretention Swale

FAIRPLAINS SCH RD

WILKES COUNTY
SENIOR CENTER

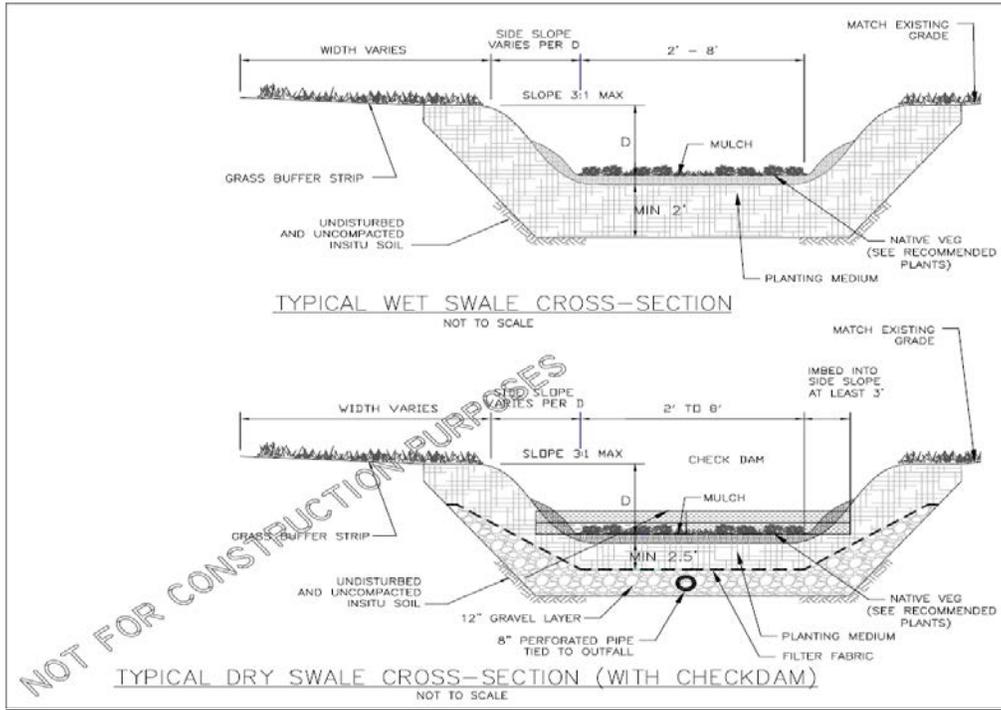
Legend:

- Parcels (white outline)
- Drainage Area (red outline)
- Roads (black line)
- Hydrology
- Source: NC Floodplain Mapping Program
- Culvert (yellow line)
- Stream / River (blue line)
- 2010 Orthophotography

1 inch = 75 feet

0 37.5 75 Feet

Diagram and photograph of a typical swale:



High Country Council of Governments
Regional Stormwater Project
Ronda



Memorial Park

Problem



The Ronda Memorial Park contains a paved parking lot and a concrete canoe launch. Runoff from the majority of the parking lot flows down a steep bank approximately 20 feet to the Yadkin River. Runoff from the parking lot, driveway, and roadway above flows down the canoe ramp directly into the Yadkin River. The runoff from the site introduces oils and grease, hydrocarbons, metals, and road salt to the stream.

Drainage area = 1.33 acre

Impervious surface = 0.50 acre; 38%

Affected stream = Yadkin River

Stream classification = WS-IV



BMP solution

A grade-level drain will be installed to capture the runoff and drain it to a channel that will connect to a bioretention cell in the grassy area. Bioretention is the preferred BMP, considering the park environment.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. The bioretention cell will also reduce erosion and sedimentation resulting from stormwater draining unchecked down the bank to the river.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	15		3		12
COD	259		U		U
TSS	665		40		625
LEAD	1		U		U
COPPER	0		U		U
ZINC	1		U		U
TDS	1,783		U		U
TN	4		U		U
TKN	6		U		U
DP	0		U		U
TP	1		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$9,950
Design & engineering	<u>\$ 500</u>
	\$10,450

Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Project Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Ronda Site: Memorial Park



TOWN OF RONDA

CLINGMAN RD

Site 1: Bioretention Cell

Yadkin River

Legend:

- Parcels
- Drainage Area
- Roads

Hydrology

Source: NC Floodplain Mapping Program

- Culvert
- Stream / River

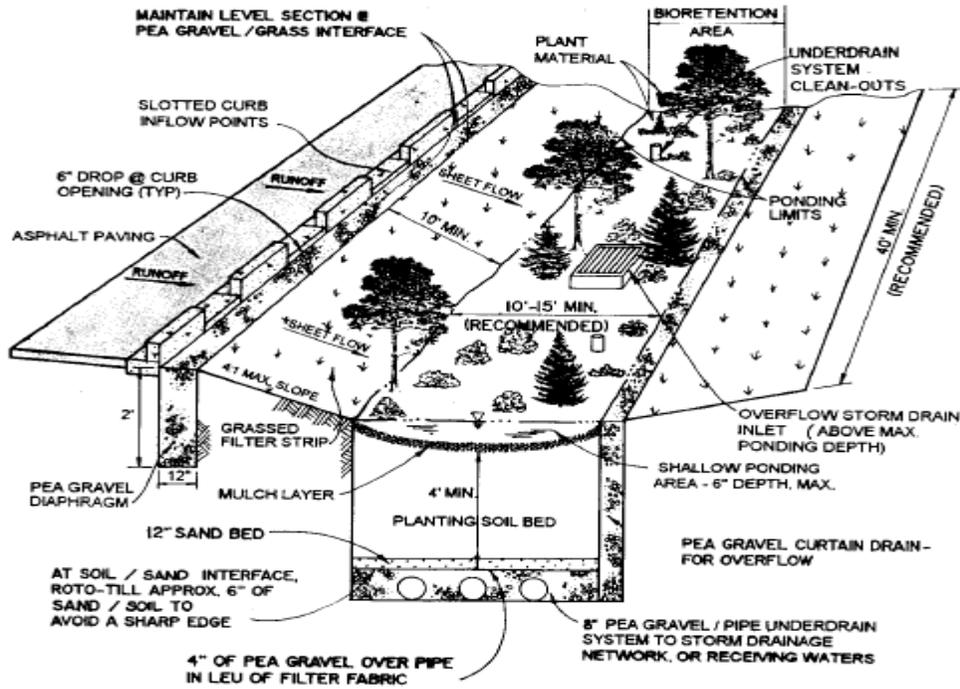
2010 Orthophotography

1 inch = 75 feet

0 37.5 75 Feet



Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Town of Wilkesboro



Tyson Plant (Main/Cherry Streets)

Problem



An unnamed tributary of Cub Creek and the stormwater from the Tyson plant and surrounding area flow down the channel shown in the right photo (flow and channel location shown by red arrow). In addition to the typical pollutants associated with parking lots, drives, and rooftops, water quality testing by the Town a few feet downstream revealed high levels of BOD, low dissolved oxygen, and high levels of fecal coliform bacteria.

Drainage area = 116.57 acres Impervious surface = 62.8 acres; 54%



Affected stream= unnamed tributary to Cub Creek
Stream classification = C

BMP solution

A weir will be installed on the existing concrete channel to divert stormflow to an adjacent parcel where a series of wetland cells will be constructed. The topography is conducive to this BMP and will provide substantial filtration of the targeted pollutants. The Town is currently engaged in a major streambank restoration effort on Cub Creek only a short distance downstream, which this BMP will complement and help protect from excessive stormwater flows.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be filtered. The BMP will remedy high levels of BOD, low dissolved oxygen, and high levels of fecal coliform bacteria currently in the stream. Elimination of the stormwater volume from the creek will lessen erosion downstream that often results from higher stream volumes.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	4,663		1,725		2,938
COD	26,811		13,406		13,406
TSS	125,896		28,327		97,569
LEAD	162		57		105
COPPER	21		U		U
ZINC	140		91		49
TDS	131,724		U		U
TN	1,399		1,119		280
TKN	466		U		U
DP	87		U		U
TP	152		85		67
CADMIUM	3		U		U

Cost estimate²

Construction \$182,236
Design & engineering \$ 9,111
 \$191,347

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Wilkesboro: Tyson Plant (Main/Cherry Streets)

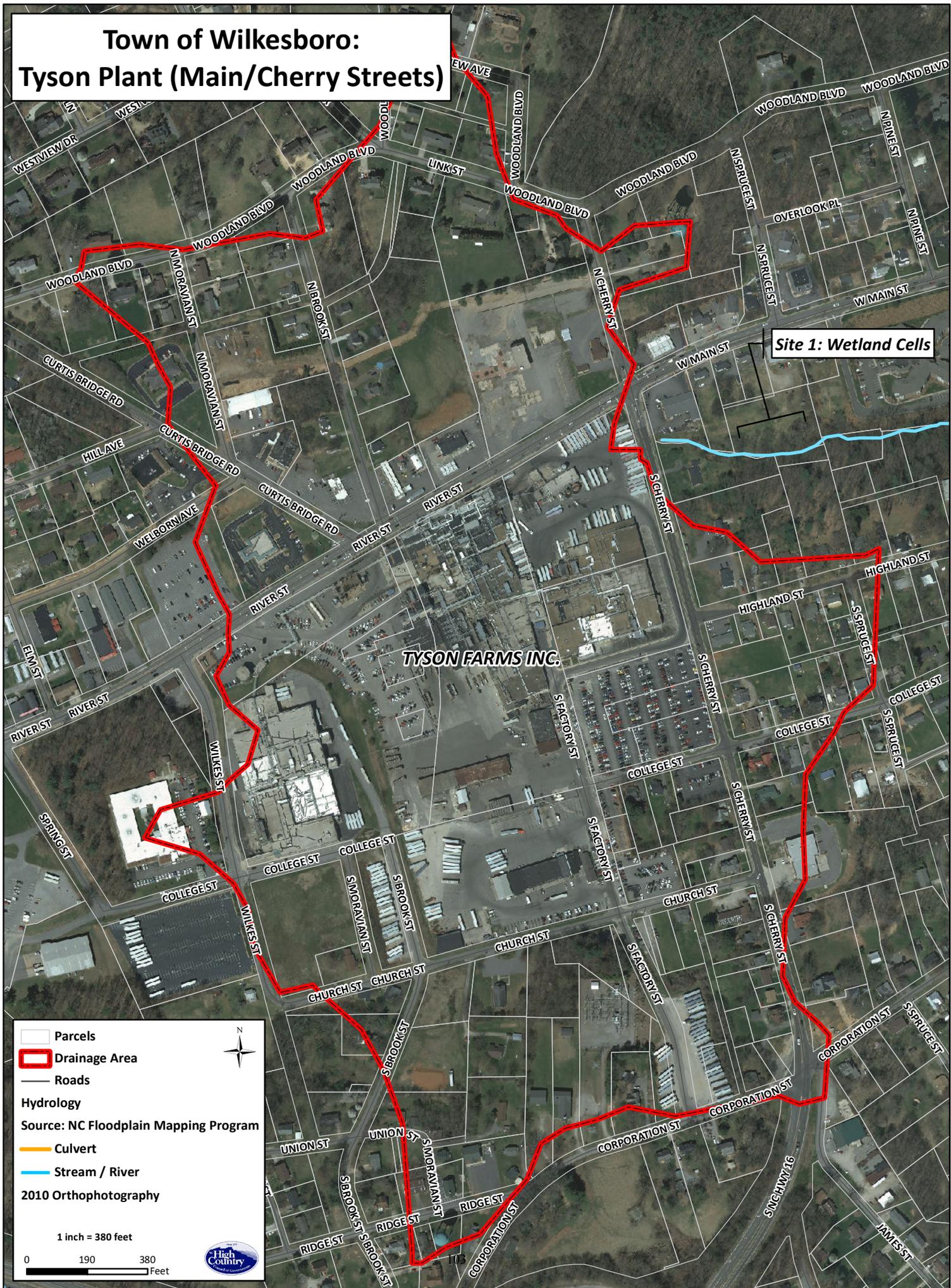
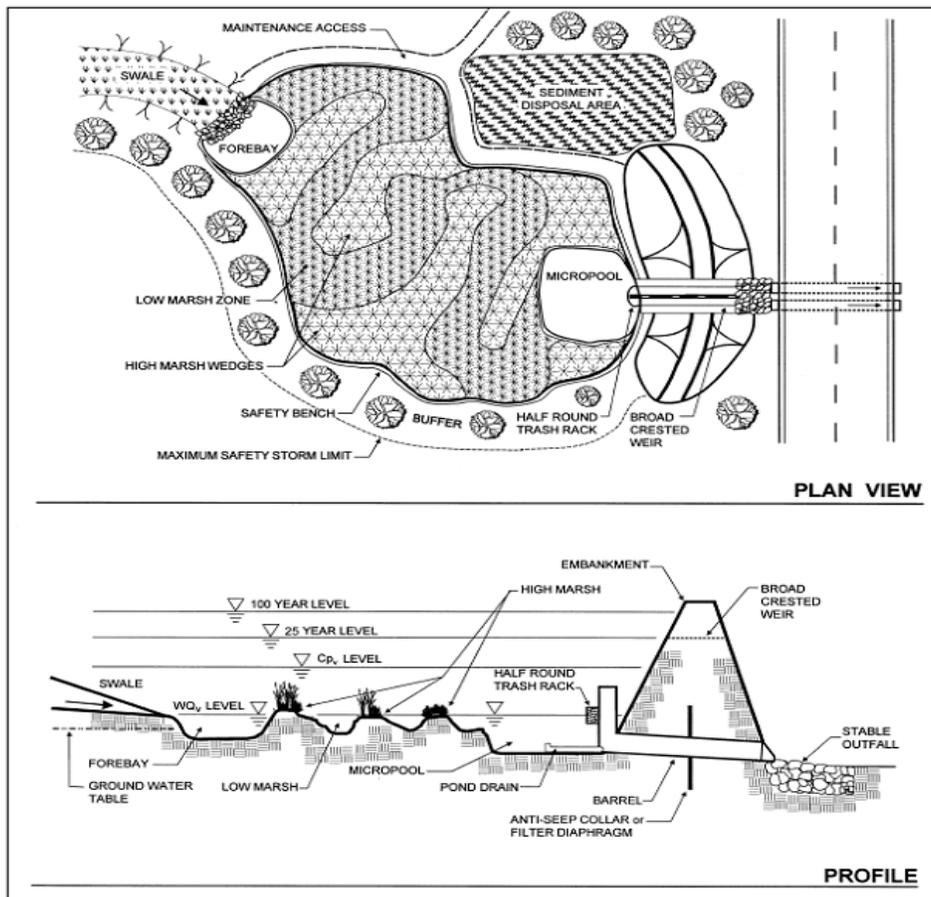


Diagram and photograph of a typical stormwater wetland:



High Country Council of Governments
Regional Stormwater Project
Town of Wilkesboro



Wilkesboro United Methodist Church

Problem



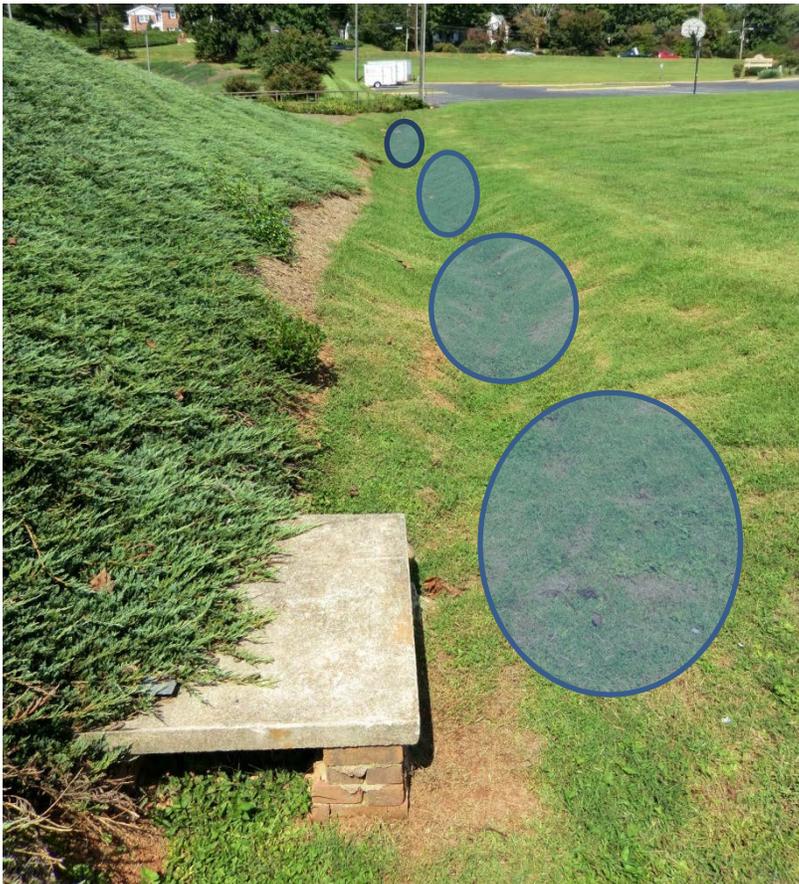
Runoff from the church parking lot flows down a drainage ditch to a pipe where it is discharged directly into a tributary of Cub Creek.

Drainage area = 0.51 acre
Impervious surface = 0.47 acre; 92%

Affected stream = unnamed tributary to Cub Creek
Stream classification = C

BMP solution

A bio-grade step or series of bioretention cells will be installed along the drainage ditch. This BMP will fit well into the existing topography and landscaping of the church grounds without excessive modification. The Town is currently engaged in a substantial streambank restoration effort on Cub Creek only a short distance downstream, which this BMP will complement and help protect from excessive flows.



Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. The BMP will also reduce velocity of stormwater discharging to the creek, reducing streambank erosion.

	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	15		3		13
COD	264		U		U
TSS	678		41		638
LEAD	1		U		U
COPPER	0		U		U
ZINC	1		U		U
TDS	1,818		U		U
TN	4		U		U
TKN	6		U		U
DP	0		U		U
TP	1		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$11,938
Design & engineering	<u>\$ 1,000</u>
	\$12,938

Funding

North Carolina Clean Water Management Trust Fund

Soil & Water District Community Conservation Assistance Project Program

North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Town of Wilkesboro Site: United Methodist Church

W MAIN ST

WILKESBORO METHODIST CHURCH INC.

Site 1: Bioretention Cells

Legend

- Parcels
- Drainage Area
- Roads

Hydrology

Source: NC Floodplain Mapping Program

- Culvert
- Stream / River

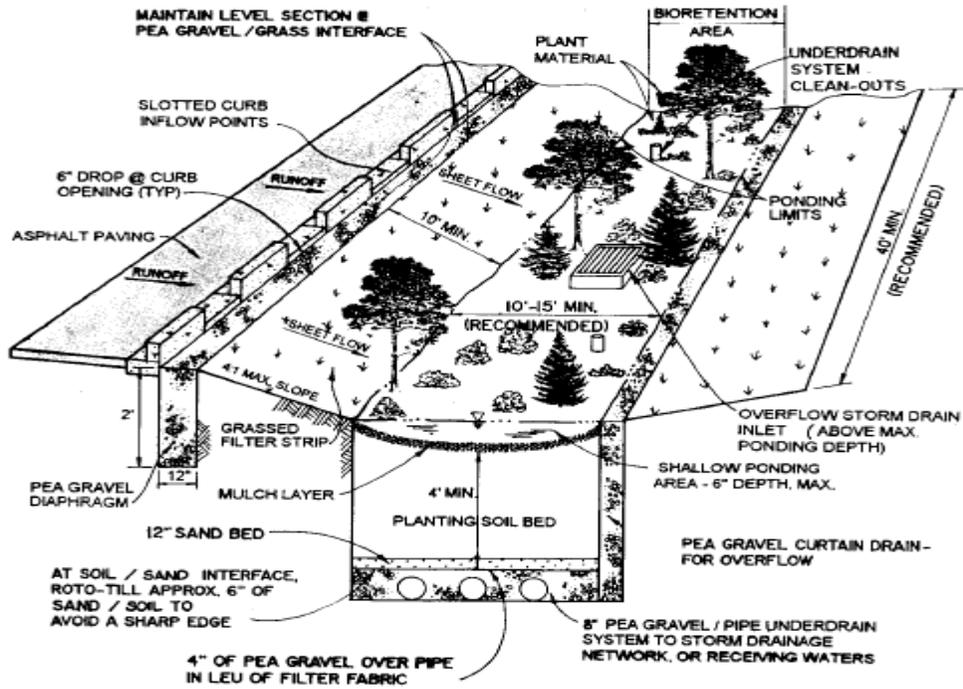
2010 Orthophotography

1 inch = 50 feet

0 25 50 Feet



Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
Regional Stormwater Project
Yancey County



Yancey County Health Department



Problem

Runoff from the Health Department parking lot and Wheeler Hills Road is directed via drains to the roadside ditch where it eventually flows into a nearby stream. The runoff is introducing pollutants, and increasing runoff velocity, to the receiving stream.

Drainage area = 0.56 acre Impervious surface = 0.37 acre; 65%

Affected stream = unnamed tributary to Cane River

Stream classification = C, Tr



BMP solution

The drainage ditch will be converted to a bioretention swale, replacing at least some of the soil with materials and a soil mix that will retain greater quantities of runoff. The addition of appropriate plants will enhance its filtering and retention capacity. A series of checkdams will slow the velocity to allow more infiltration. Bioretention will achieve water quality objectives while maintaining a landscaped appearance at the entrance to the agency.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Slowing velocity of the runoff to neighboring properties will reduce sedimentation. This BMP's impact in the Cane River watershed will augment improvements currently underway on the Town of Burnsville's wastewater treatment system.

bioretention ¹	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	49		24		25
COD	844		507		338
TSS	2,168		585		1,583
LEAD	3		1		1
COPPER	1		U		U
ZINC	3		1		2
TDS	5,811		U		U
TN	13		8		5
TKN	18		U		U
DP	0		U		U
TP	2		1		1
CADMIUM	0		U		U

Cost estimate²

Construction	\$13,975
Design & engineering	\$ 1,300
	\$15,275

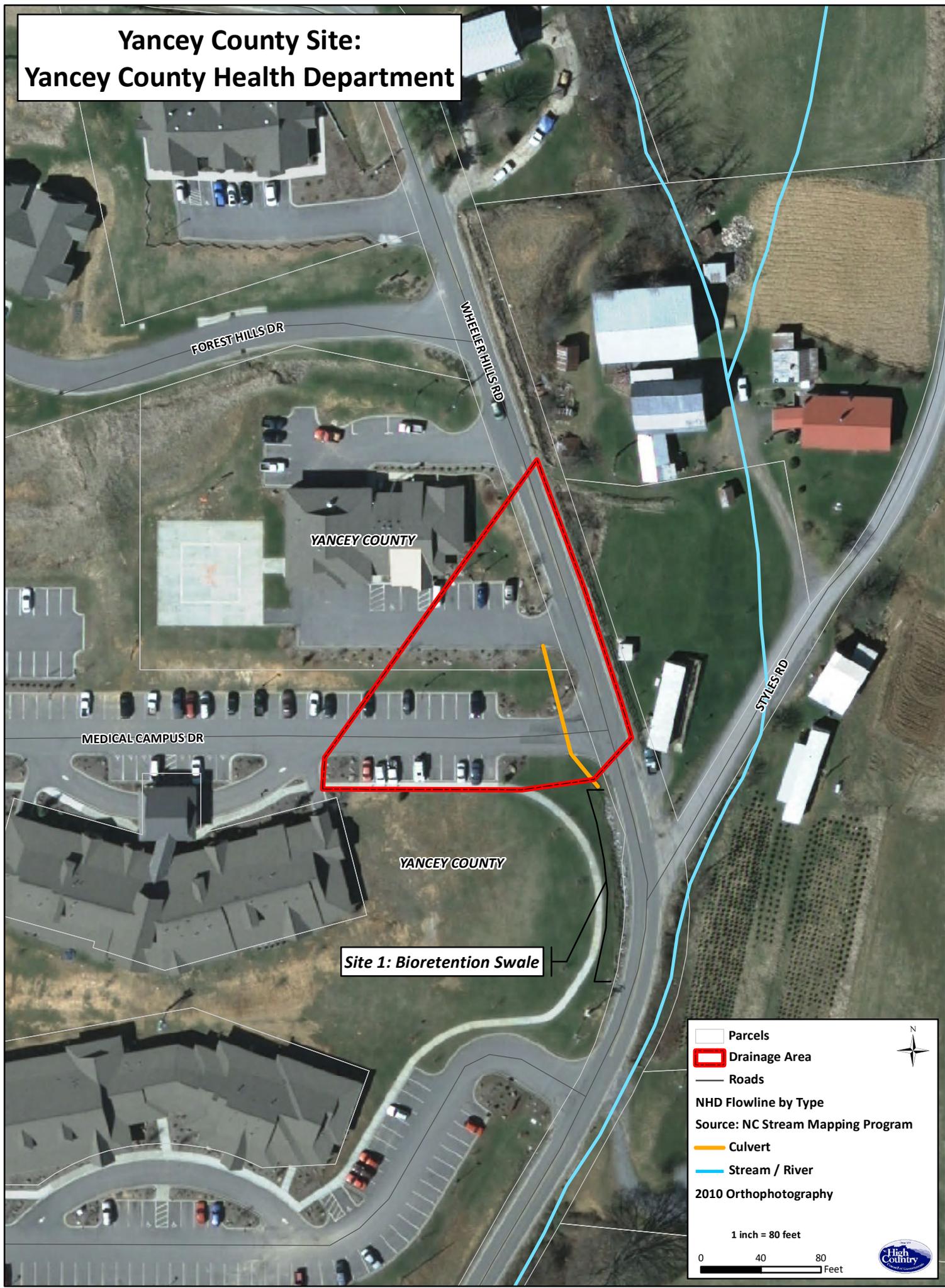
Funding

NC Clean Water Management Trust Fund
 Soil & Water District Community Conservation Assistance Program
 North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Yancey County Site: Yancey County Health Department



Site 1: Bioretention Swale

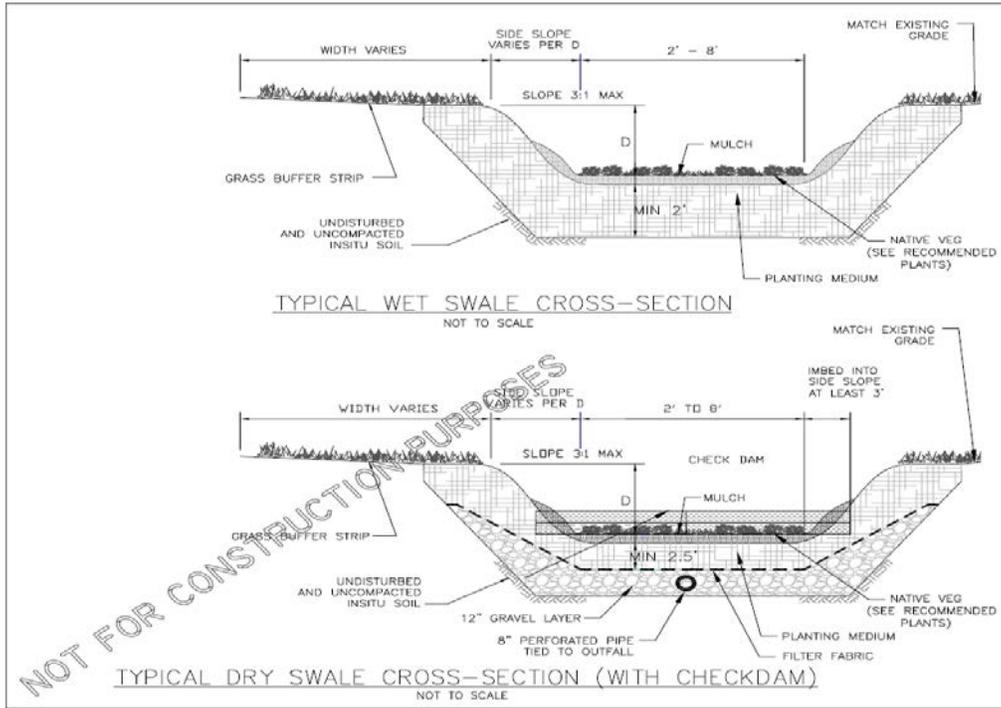
Legend:

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 80 feet

0 40 80 Feet

Diagram and photograph of a typical swale:



High Country Council of Governments
Regional Stormwater Project
Yancey County



Ray Cort Park

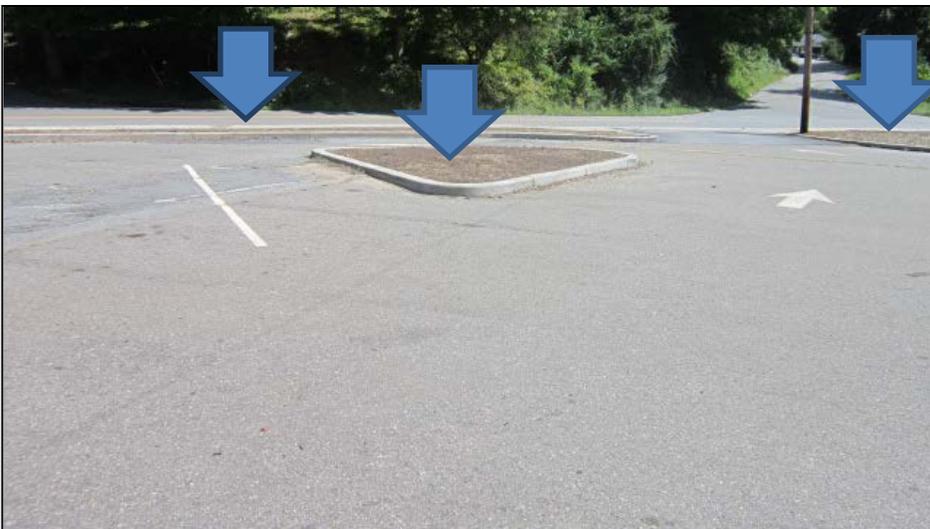
Problem

Runoff from the Ray Cort Park parking lot drains directly to the stream that flows parallel to the lot.



Drainage area = 0.40 acre
Impervious surface = 0.35 acre; 88%

Affected stream = Mitchell Branch
Stream classification = C, Tr



BMP solution

Converting the existing planter islands (indicated by arrows) into bioretention cells will achieve water quality objectives while maintaining aesthetic qualities, minimizing costs, and effectively dealing with the lack of space for a BMP. The BMP will also serve an educational purpose at this public park with appropriate interpretive signage installed.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots will be prevented from entering the stream. Reduction of the stormwater velocity discharging to the creek will help lessen erosion and sedimentation downstream that often results from higher stream volumes. Temperature fluctuations caused by this site's runoff will be eliminated.

bioretention	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	11		5		6
COD	192		115		77
TSS	492		133		359
LEAD	1		0		0
COPPER	0		U		U
ZINC	1		0		0
TDS	1,319		U		U
TN	3		2		1
TKN	4		U		U
DP	0		U		U
TP	0		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$7,000
Design & engineering	<u>\$ 700</u>
	\$7,700

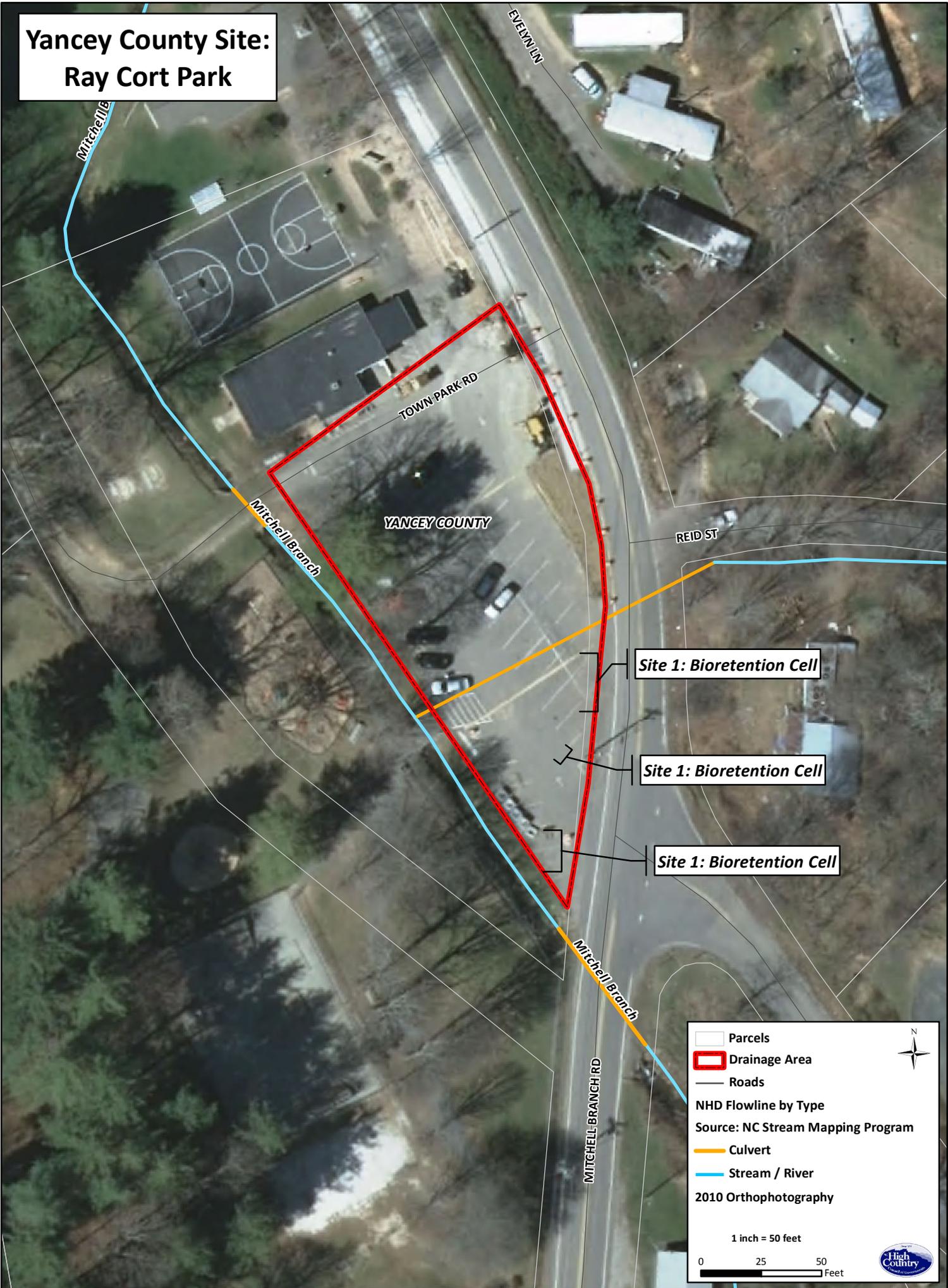
Funding

North Carolina Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

Yancey County Site: Ray Cort Park



Legend

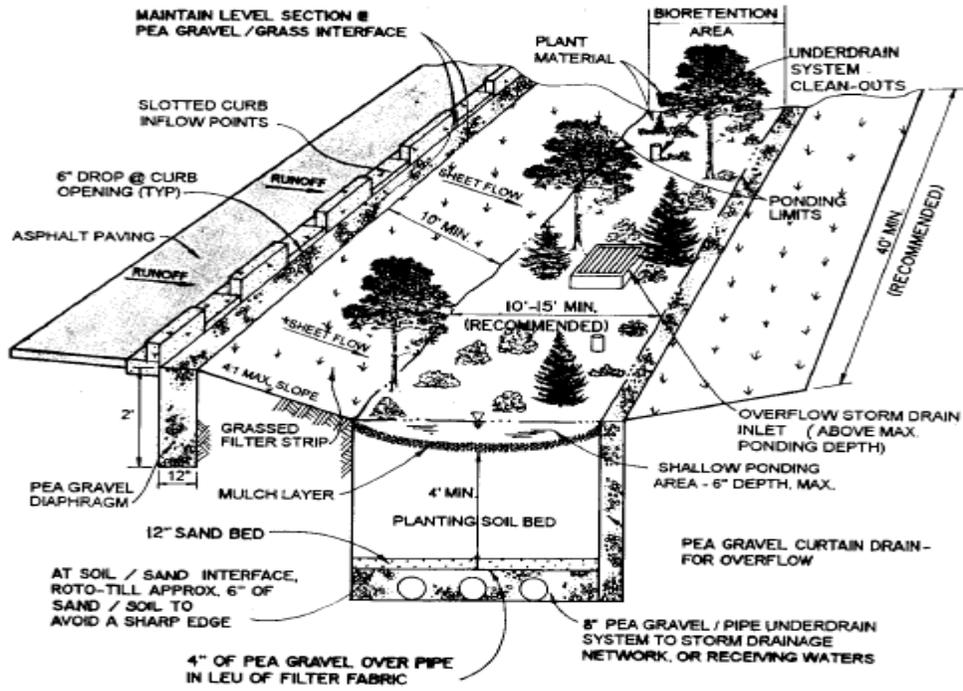
- Parcels
- Drainage Area**
- Roads
- NHD Flowline by Type**
Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 50 feet

0 25 50 Feet



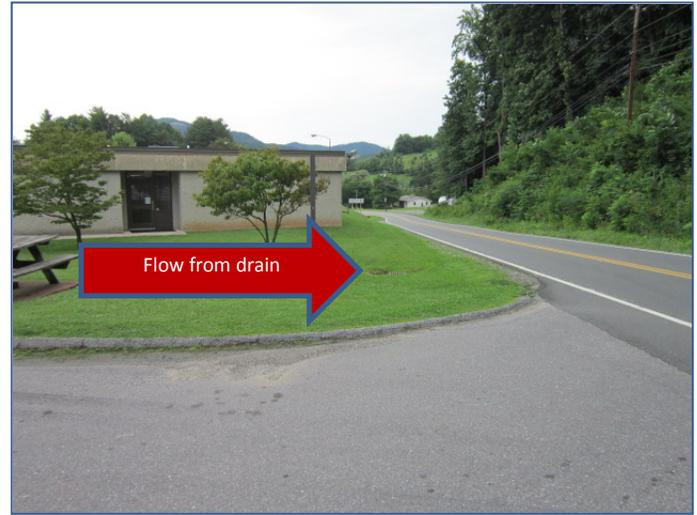
Diagram and photograph of a typical bioretention cell:



High Country Council of Governments
 Regional Stormwater Project
 Town of Burnsville



Yancey County Department of Social Services



Problem

A significant amount of the site's runoff, primarily from the parking lot, is directed to a ditch in front of the building, where it flows into a stream. The ditch also captures runoff from the road, which includes salt in the winter. The runoff from the pavement introduces oils and grease, hydrocarbons, metals, and road salt to the receiving stream.



Drainage area = 1.63 acre
 Impervious surface = 0.77 acre; 47%

Affected stream = unnamed tributary to Little Crabtree Creek
 Stream classification = C, Tr

BMP solution

The drainage ditch's existing form will be used to convert it to a bioretention swale, replacing at least some of the soil with materials and a soil mix that will retain greater quantities of runoff. Space is available to widen it to increase its volume. The addition of appropriate plants will also enhance its filtering and retention capacity, and give this BMP a landscaped appearance that is needed at this highly visible location. A series of checkdams will slow the velocity to allow more infiltration.

Water quality benefits¹

By capturing and treating the runoff, heavy metals and other pollutants associated with parking lots and roadways will be prevented from entering the stream. Reduction or elimination of the stormwater volume entering the creek will mitigate erosion associated with higher stream volumes.

Vegetated swale ¹	Load before BMP (lbs/yr)		Load after BMP (lbs/yr)		Load Reduction (lbs/yr)
BOD	17		8		8
COD	290		174		116
TSS	745		201		544
LEAD	1		0		0
COPPER	0		U		U
ZINC	1		0		1
TDS	1,996		U		U
TN	4		3		2
TKN	6		U		U
DP	0		U		U
TP	1		0		0
CADMIUM	0		U		U

Cost estimate²

Construction	\$6,715
Design & engineering	<u>\$ 600</u>
	\$7,315

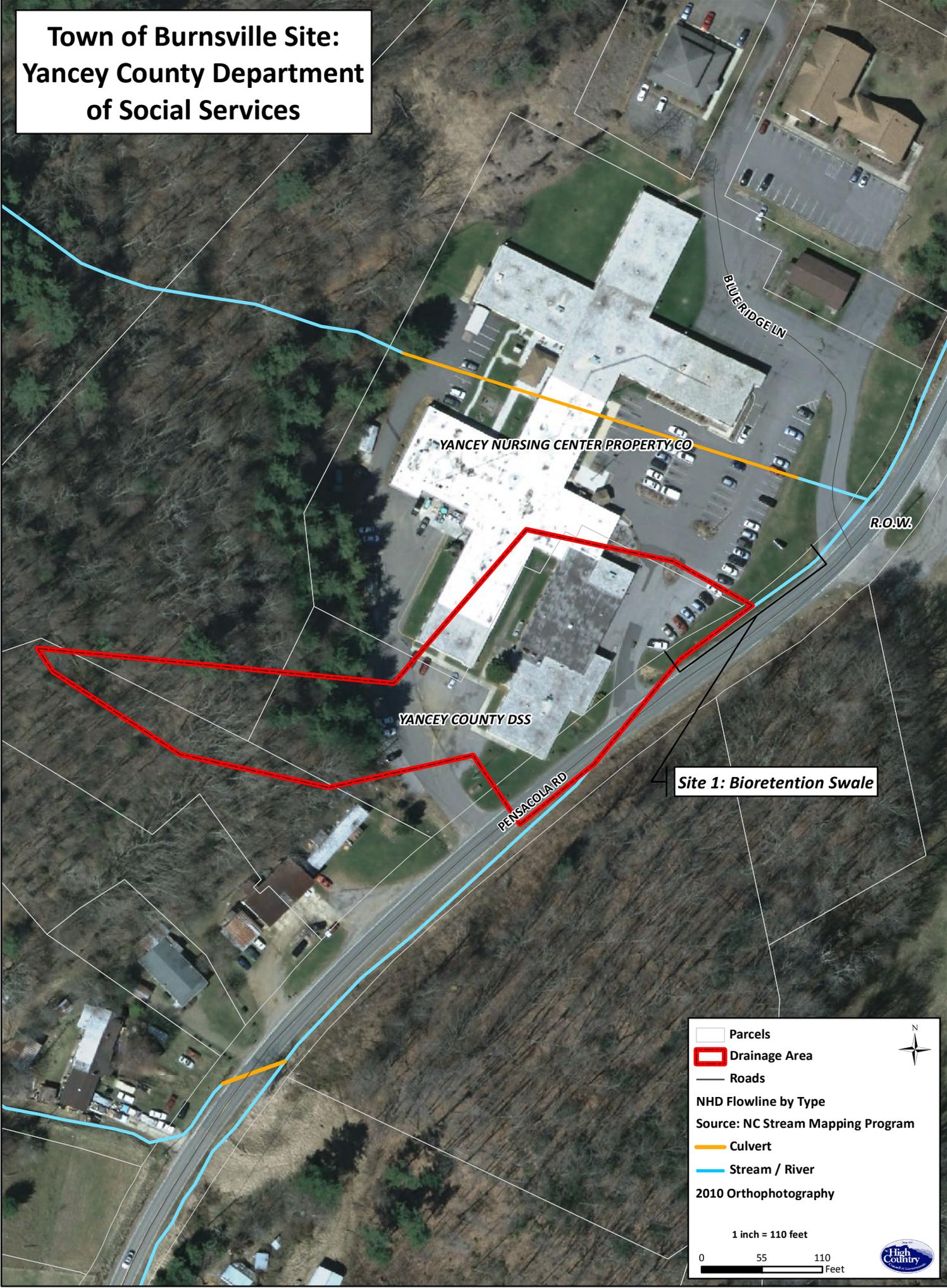
Funding

NC Clean Water Management Trust Fund
Soil & Water District Community Conservation Assistance Program
North Carolina Division of Water Resources Development Project Grant Program

¹ The methodology used to obtain values was developed by the Illinois Environmental Protection Agency. U = unavailable

² Methodology used to calculate cost estimates is obtained from *Urban Subwatershed Restoration Manual No. 3 URBAN STORMWATER RETROFIT PRACTICES Version 1.0* Tom Schueler, David Hirschman, Michael Novotney, and Jennifer Zielinski, Center for Watershed Protection, 2007

**Town of Burnsville Site:
Yancey County Department
of Social Services**



Site 1: Bioretention Swale

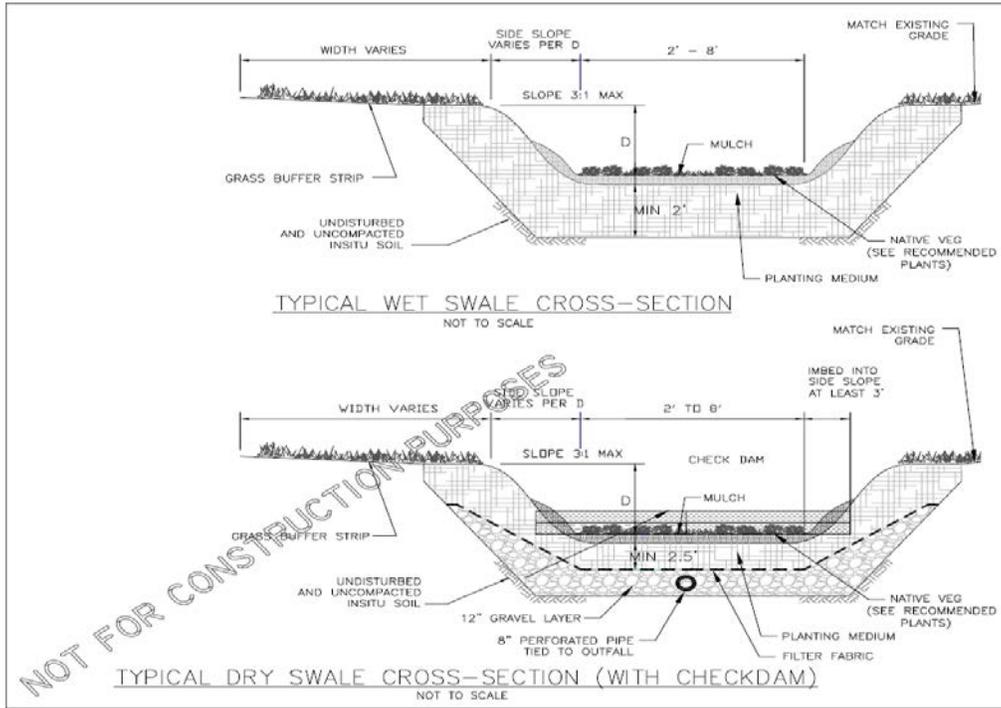
Legend:

- Parcels
- Drainage Area
- Roads
- NHD Flowline by Type
- Source: NC Stream Mapping Program
- Culvert
- Stream / River
- 2010 Orthophotography

1 inch = 110 feet

0 55 110 Feet

Diagram and photograph of a typical swale:



MODEL
**LOCAL GOVERNMENT STORMWATER
MANAGEMENT ORDINANCE**

PURPOSE

The purpose of this ordinance is to protect, maintain and enhance the public health, safety, environment and general welfare by establishing minimum requirements and procedures to control the adverse effects of increased post-development stormwater runoff and nonpoint and point source pollution associated with new development and redevelopment as well as illicit discharges into municipal stormwater systems. It has been determined that proper management of construction-related and post-development stormwater runoff will minimize damage to public and private property and infrastructure; safeguard the public health, safety, and general welfare; and protect water and aquatic resources.

APPLICABILITY AND JURISDICTION

- (A) The provisions of this ordinance shall apply within the Town Limits and any Extraterritorial Jurisdiction (ETJ).
- (B) The following development activities are exempt from the provisions of this ordinance:
 - (1) Construction of a single-family or two-family residence
 - (2) Redevelopment, or change in use of a structure, that does not involve more than 5,000 square feet of land disturbance
 - (3) Redevelopment, or change in use of a structure, that does not involve construction of more than 5,000 square feet of additional impervious surface
 - (4) Agriculture and forestry practices

RELATIONSHIP TO OTHER LAWS, REGULATIONS AND PRIVATE AGREEMENTS

This ordinance is not intended to modify or repeal any other ordinance, rule, regulation or other provision of law. The requirements of this ordinance are in addition to the requirements of any other ordinance, rule, regulation or other provision of law. Where any provision of this ordinance imposes restrictions different from those imposed by any other ordinance, rule, regulation or other provision of law, whichever provision is more restrictive or imposes higher protective standards for human or environmental health, safety, and welfare shall control.

EFFECTIVE DATE AND TRANSITIONAL PROVISIONS

- (A) This Ordinance shall take effect on _____, 201__.
- (B) All development and redevelopment projects for which all necessary permits were issued prior to the effective date of this ordinance and which remain valid, unexpired, unrevoked and not otherwise terminated at the time of development or redevelopment shall be exempt from complying with all provisions of this ordinance dealing with the control and/or management of post-construction runoff, but shall be required to comply with all other applicable provisions (including but not limited to illicit discharge provisions).

DRAINAGE PLAN REQUIRED

Any development or redevelopment subject to the provisions of this ordinance shall submit a Drainage Plan for review by the Town in conjunction with application for a Zoning Permit. The Town shall review the Drainage Plan for compliance with the provisions of this ordinance. The Drainage Plan shall be prepared and bear the seal of a licensed professional engineer, and include the following:

- (A) Topographic Map of the total drainage area that includes the project site. The Topographic Map shall have a scale not smaller than 1 inch = 50 feet, and include the following:
 - (1) Contours at two-foot intervals
 - (2) Property lines
 - (3) Project construction elements (structures, parking lots, driveways, and other impervious surfaces)
 - (4) Existing perennial and intermittent streams; wetlands; and springs
 - (5) Existing man-made stormwater facilities
- (B) Engineering drawings depicting design and details of proposed piping, drainage structures, retention/detention structures, and channels connecting to a network of man-made or natural drainage features
- (C) Specifications of piping, drainage structures, permanent erosion control measures, and retention/detention structures
- (D) Computations to support the design and specifications

GENERAL PROVISIONS

- (A) To the extent practical, lot lines in subdivisions shall follow natural and existing man-made drainage features
- (B) Stormwater shall not be channeled into a sanitary sewer system

DEVELOPMENT STANDARDS

- (A) Stormwater runoff from the development shall be transported from the development by vegetated conveyances to the maximum extent practicable.
- (B) All built-upon area shall be at a minimum of 30 feet landward of all perennial and intermittent surface waters.
- (C) Post-development runoff rate shall not exceed pre-development runoff rate. Runoff rates must be based on the same calculation method.
- (D) Stormwater management facilities shall have a minimum design capacity of the 10-year discharge. The design capacity for cross drainage facilities in public streets shall be the 25-year discharge.
- (E) Stormwater management facilities shall be designed in accordance with 15A NCAC 2H .1008
- (F) Stormwater management facilities shall be permanent, shall be protected with easements or covenants that run with the land, and shall be provided with public access

(G) Projects that drain to Class Tr waters shall include stormwater management measures that do not result in a sustained increase in water temperature of the receiving stream

(H) Minimum pipe diameter shall be 18 inches for open-ended culverts and 15 inches for closed systems and driveway culverts. Minimum pipe diameter for portions of closed systems outside the public right-of-way shall be 12 inches.

MAINTENANCE

The owner (or other responsible party) of each stormwater management facility installed pursuant to this ordinance shall maintain and operate it so as to preserve and continue its function in controlling stormwater quality and quantity at the degree or amount of function for which the facility was designed.

PERFORMANCE SECURITY FOR INSTALLATION AND MAINTENANCE

The Town may, at its discretion, require the submittal of a performance security or bond with surety, cash escrow, letter of credit or other acceptable legal arrangement prior to issuance of a permit in order to ensure that the stormwater management facilities are:

- (A) Installed as indicated in the Drainage Plan
- (B) Maintained by the owner as required by this ordinance

REMEDIES AND PENALTIES

(A) The remedies and penalties provided for violations of this ordinance, whether civil or criminal, shall be cumulative and in addition to any other remedy provided by law, and may be exercised in any order.

(B) The Town may refuse to issue a certificate of occupancy for the building or other improvements constructed or being constructed on the site and served by the stormwater practices in question until the applicant or other responsible person has taken the remedial measures set forth in the notice of violation or has otherwise cured the violations described therein.

(C) As long as a violation of this ordinance continues and remains uncorrected, the Town may disapprove any request for permit or development approval or authorization on the land on which the violation occurs.

(D) If the violation is deemed dangerous or prejudicial to the public health or public safety and is within the geographic limits prescribed by North Carolina G.S. § 160A-193, the Town may cause the violation to be corrected and the costs to be assessed as a lien against the property.

(E) The Town may issue a stop work order to the person(s) violating this ordinance. The stop work order shall remain in effect until the person has taken the remedial measures set forth in the notice of violation or has otherwise cured the violation or violations described therein. The stop work order may be withdrawn or modified to enable the person to take the necessary remedial measures to cure such violation or violations.

(F) Violation of this ordinance may be enforced as a misdemeanor subject to the maximum fine permissible under North Carolina law.

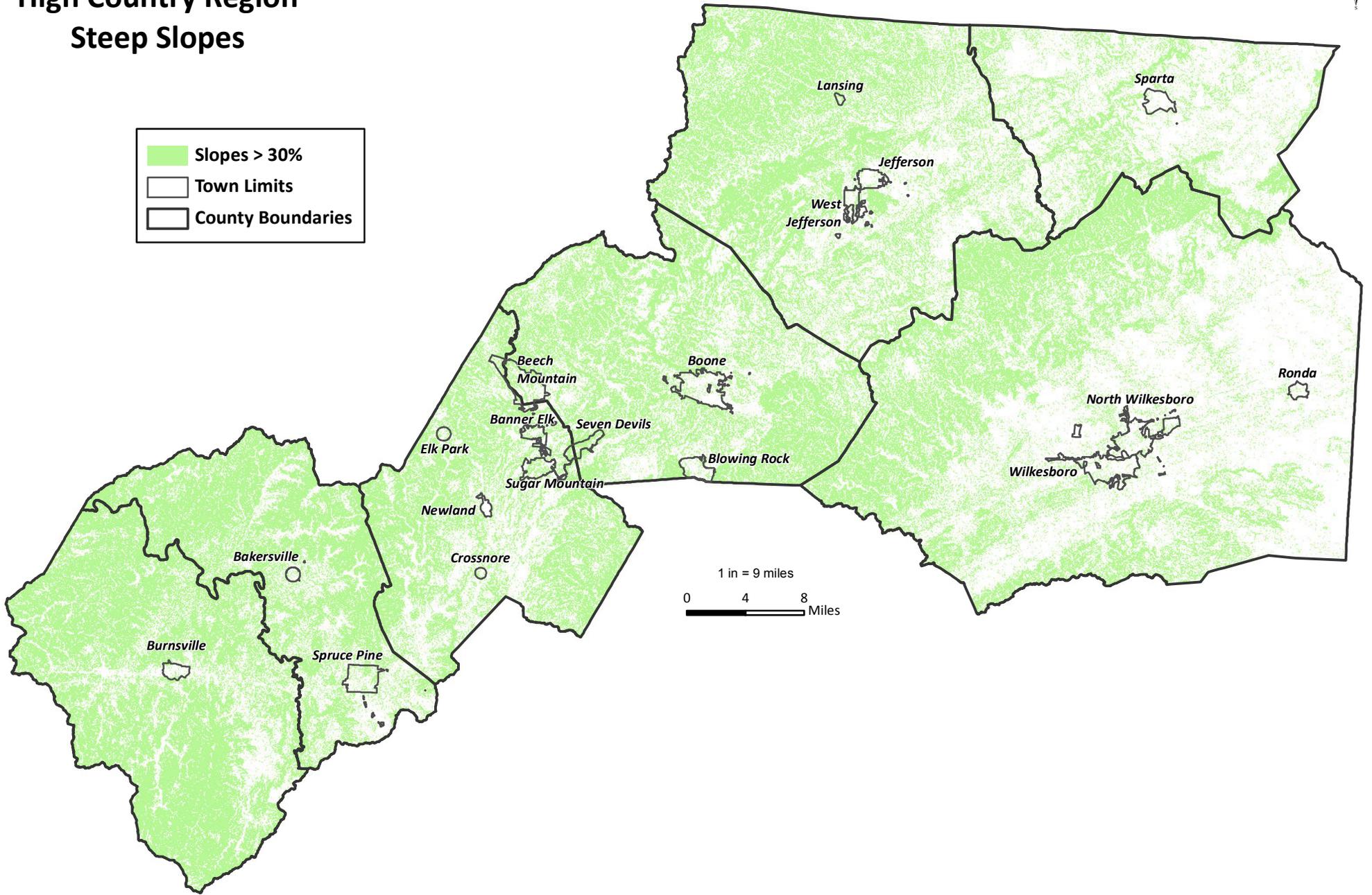
High Country Region Steep Slopes



 Slopes > 30%

 Town Limits

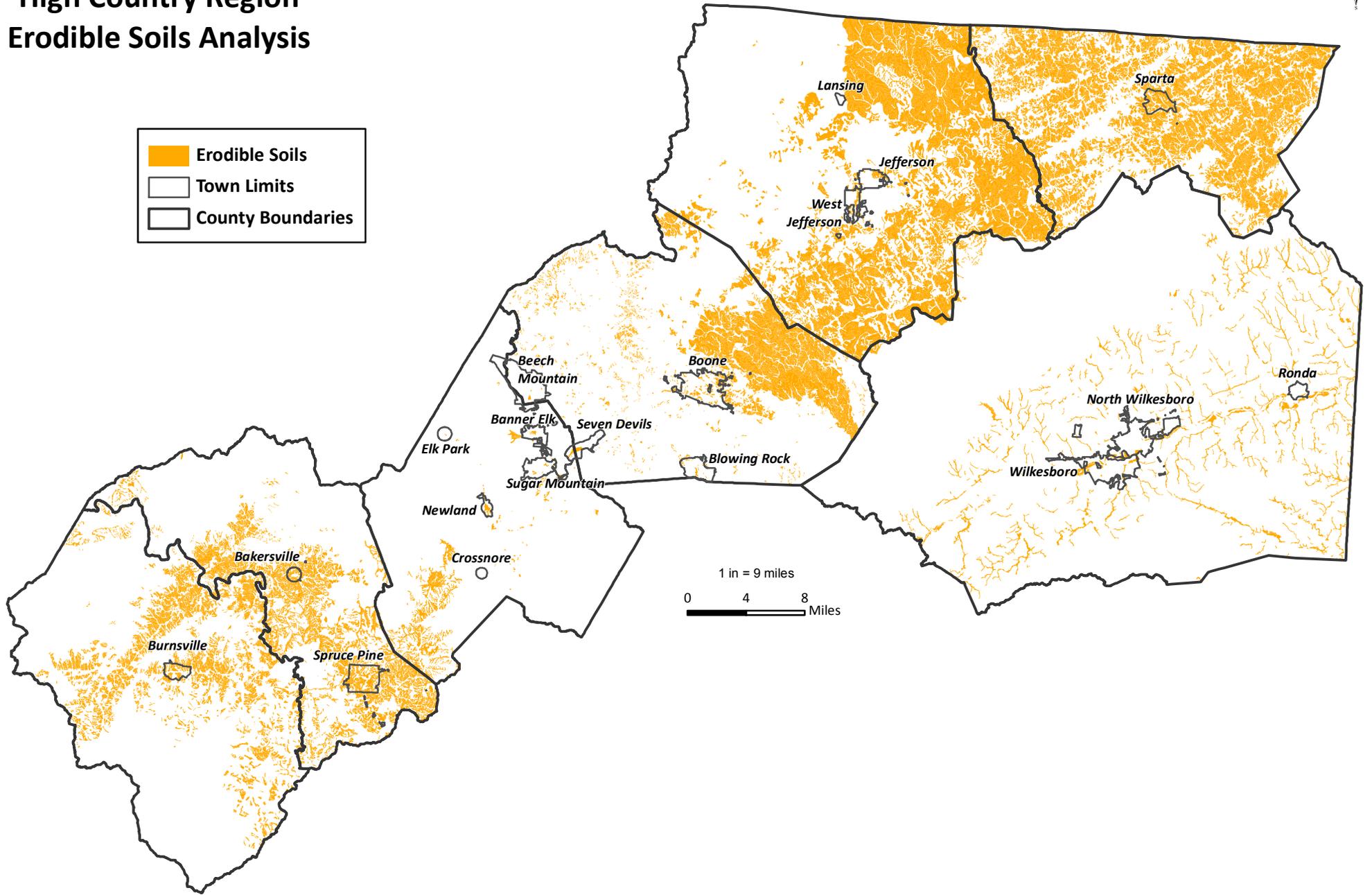
 County Boundaries



High Country Region Erodible Soils Analysis



 Erodible Soils
 Town Limits
 County Boundaries

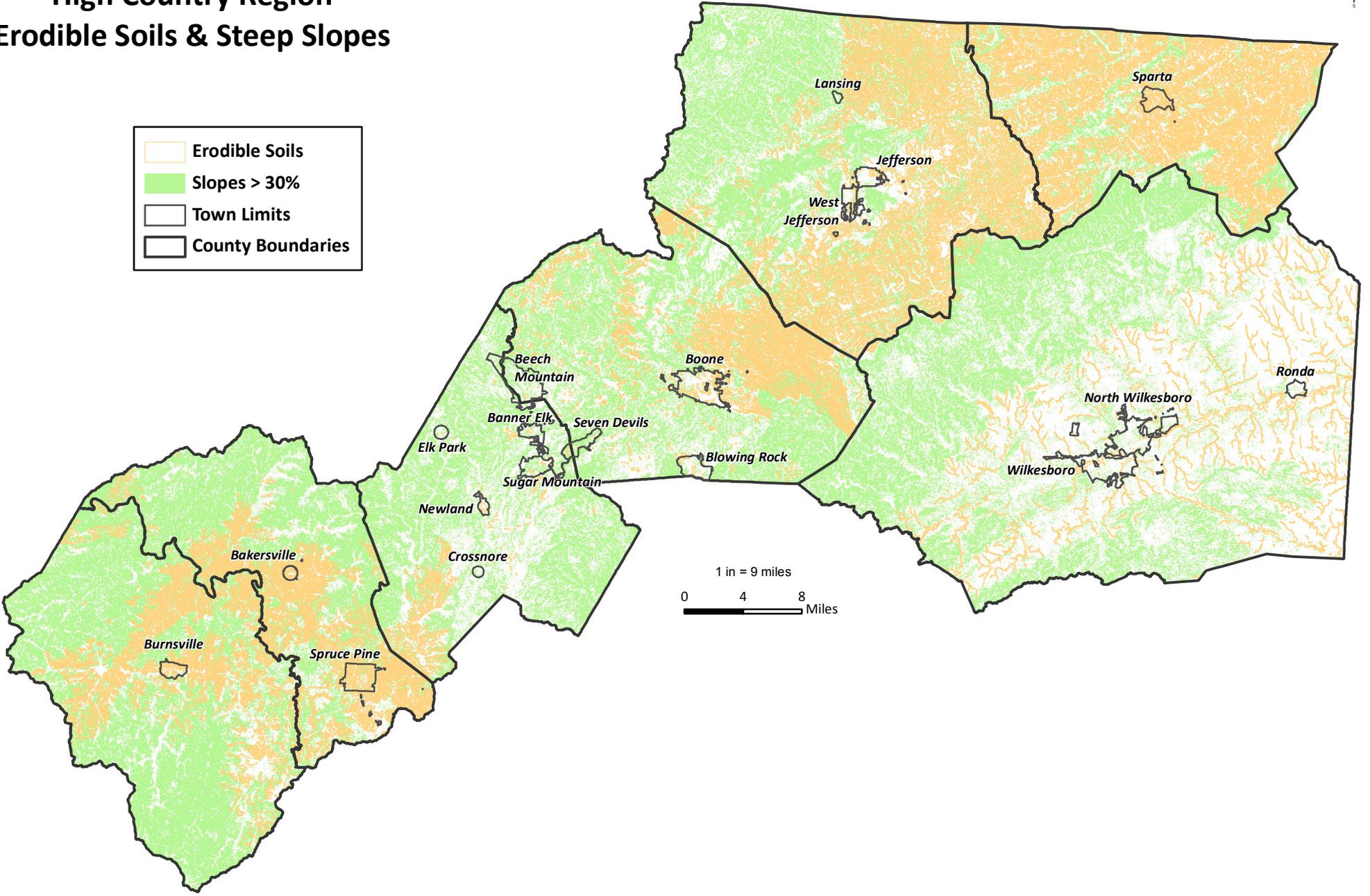


Source: NCRS SSURGO Soil Data.
Soils with a high K factor were extracted to display erodible soils based solely on surface soil texture.

High Country Region Erodible Soils & Steep Slopes



- Erodible Soils
- Slopes > 30%
- Town Limits
- County Boundaries

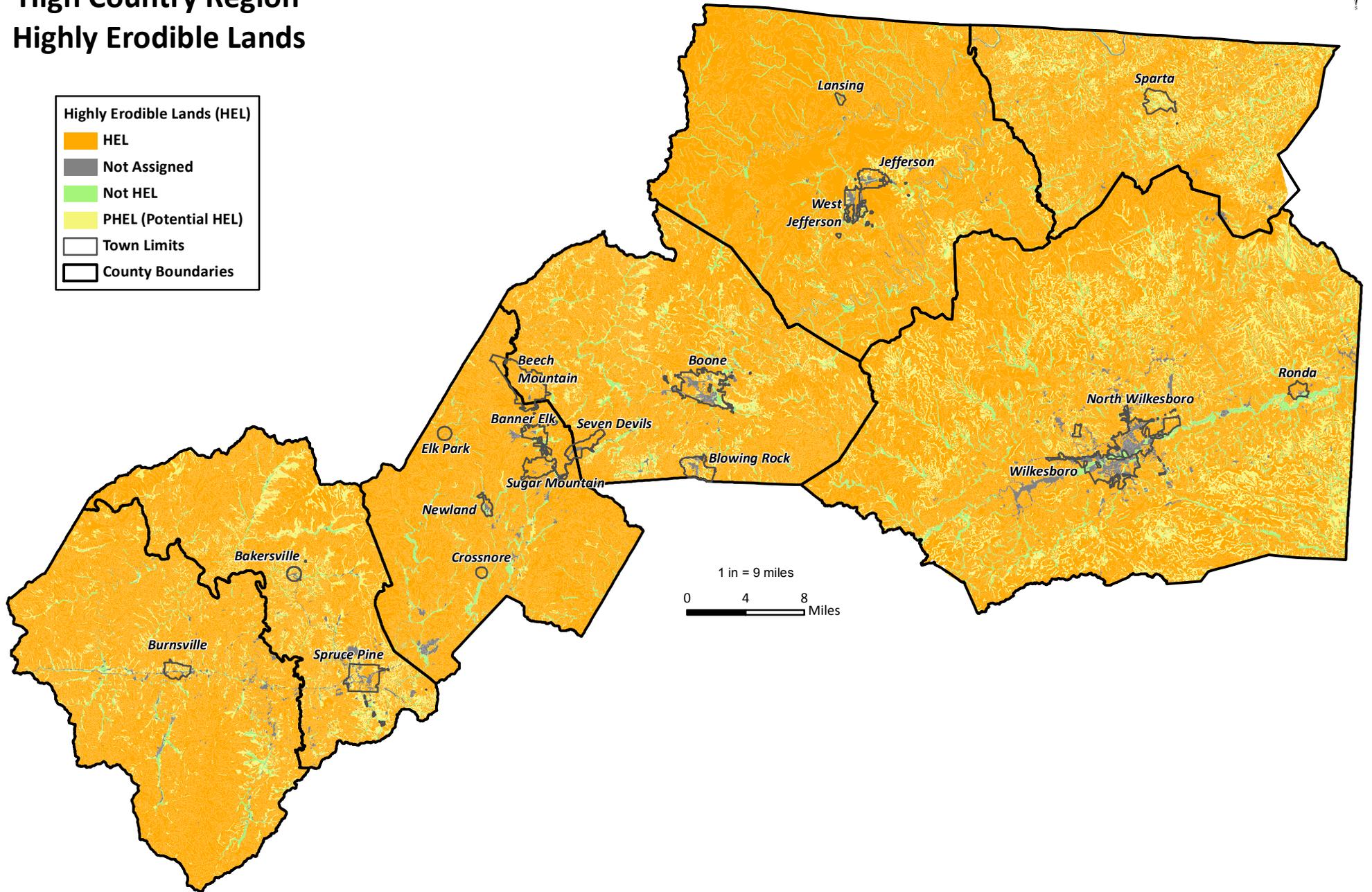


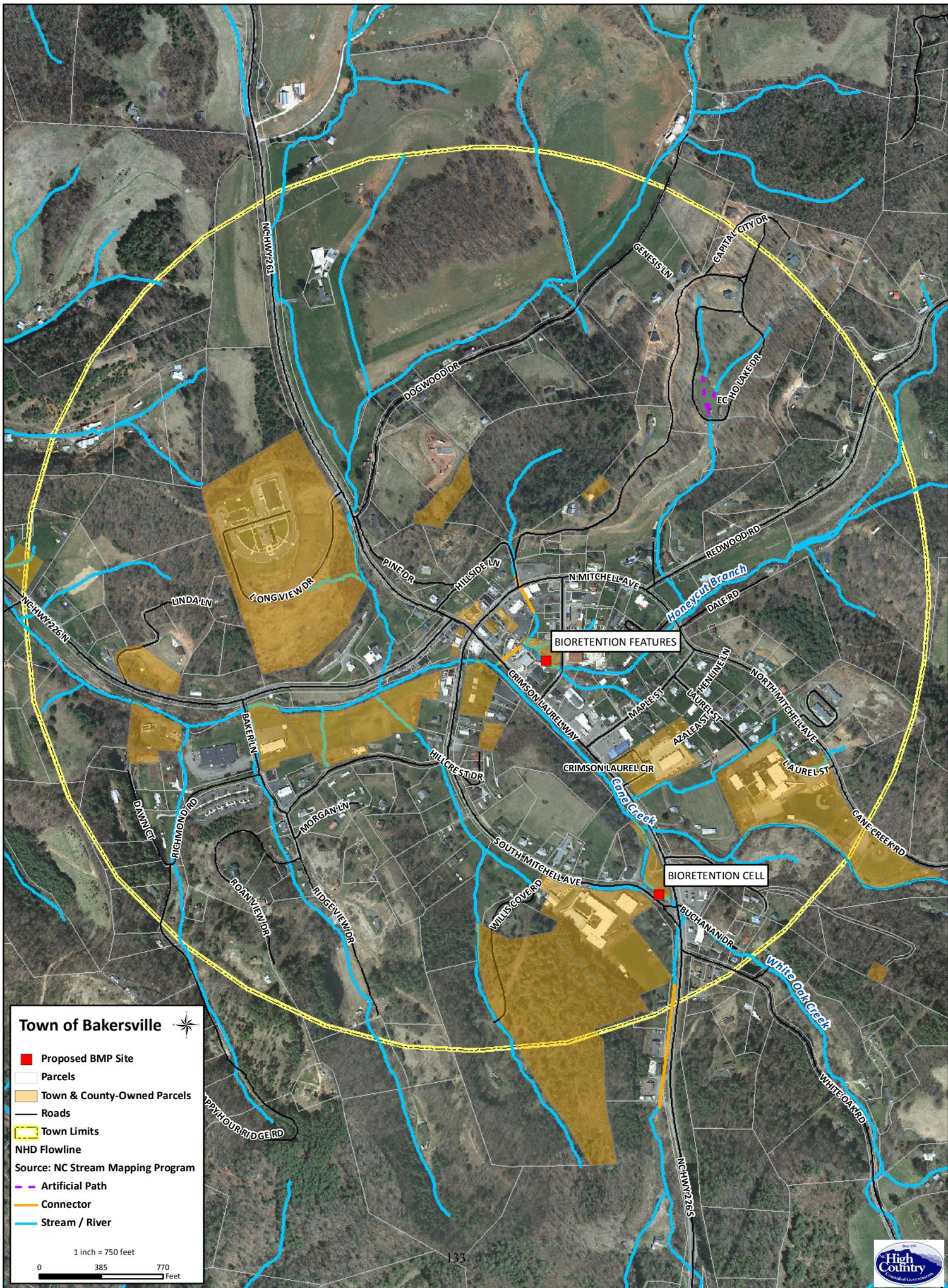
High Country Region Highly Erodible Lands



Highly Erodible Lands (HEL)

- HEL
- Not Assigned
- Not HEL
- PHEL (Potential HEL)
- Town Limits
- County Boundaries





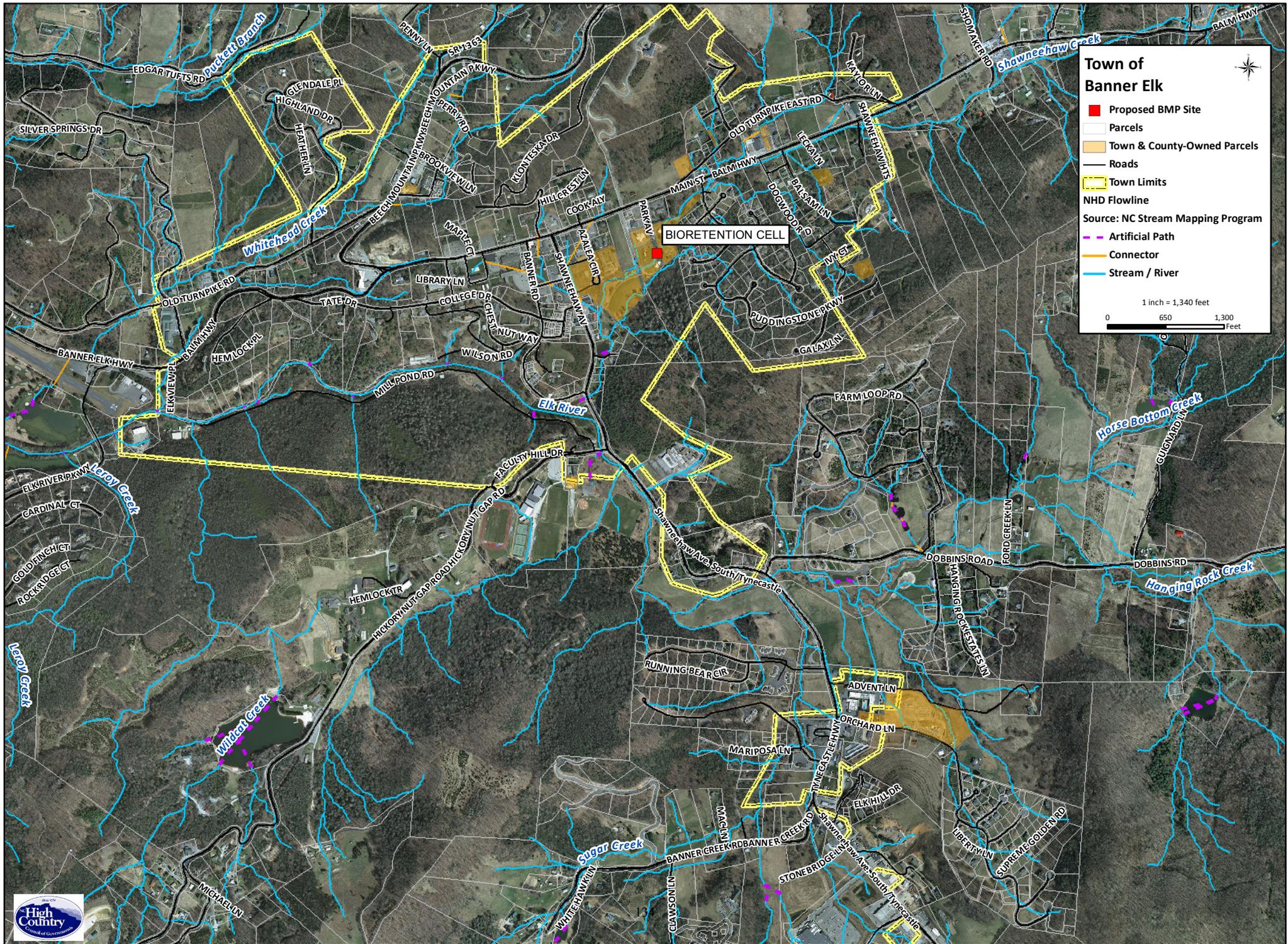
Town of Bakersville 

-  Proposed BMP Site
-  Parcels
-  Town & County-Owned Parcels
-  Roads
-  Town Limits
-  NHD Flowline
- Source: NC Stream Mapping Program
-  Artificial Path
-  Connector
-  Stream / River

1 inch = 750 feet

0 385 770 Feet





Town of Banner Elk

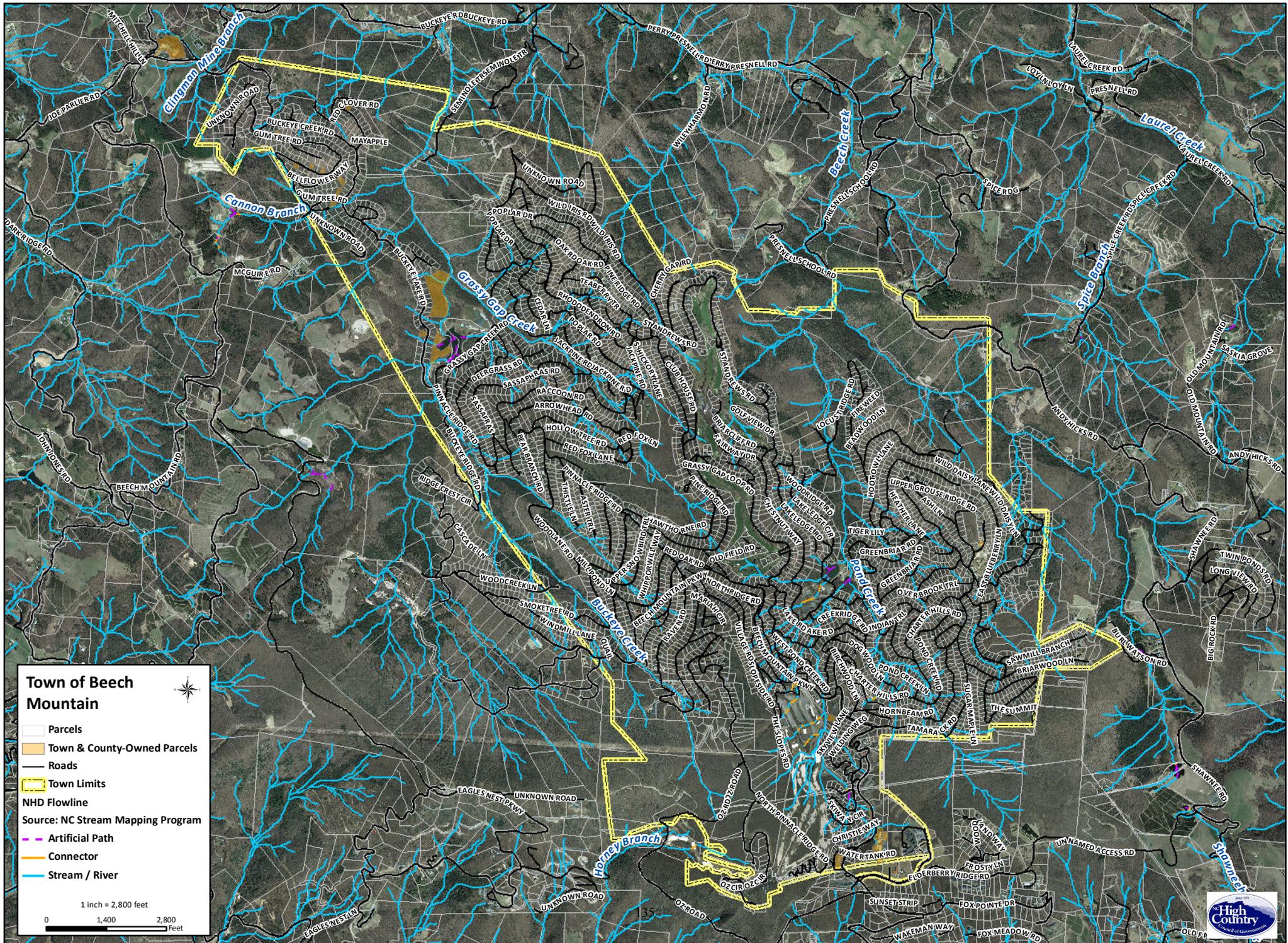
- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Artificial Path
- Connector
- Stream / River

Source: NC Stream Mapping Program

1 inch = 1,340 feet

0 650 1,300 Feet





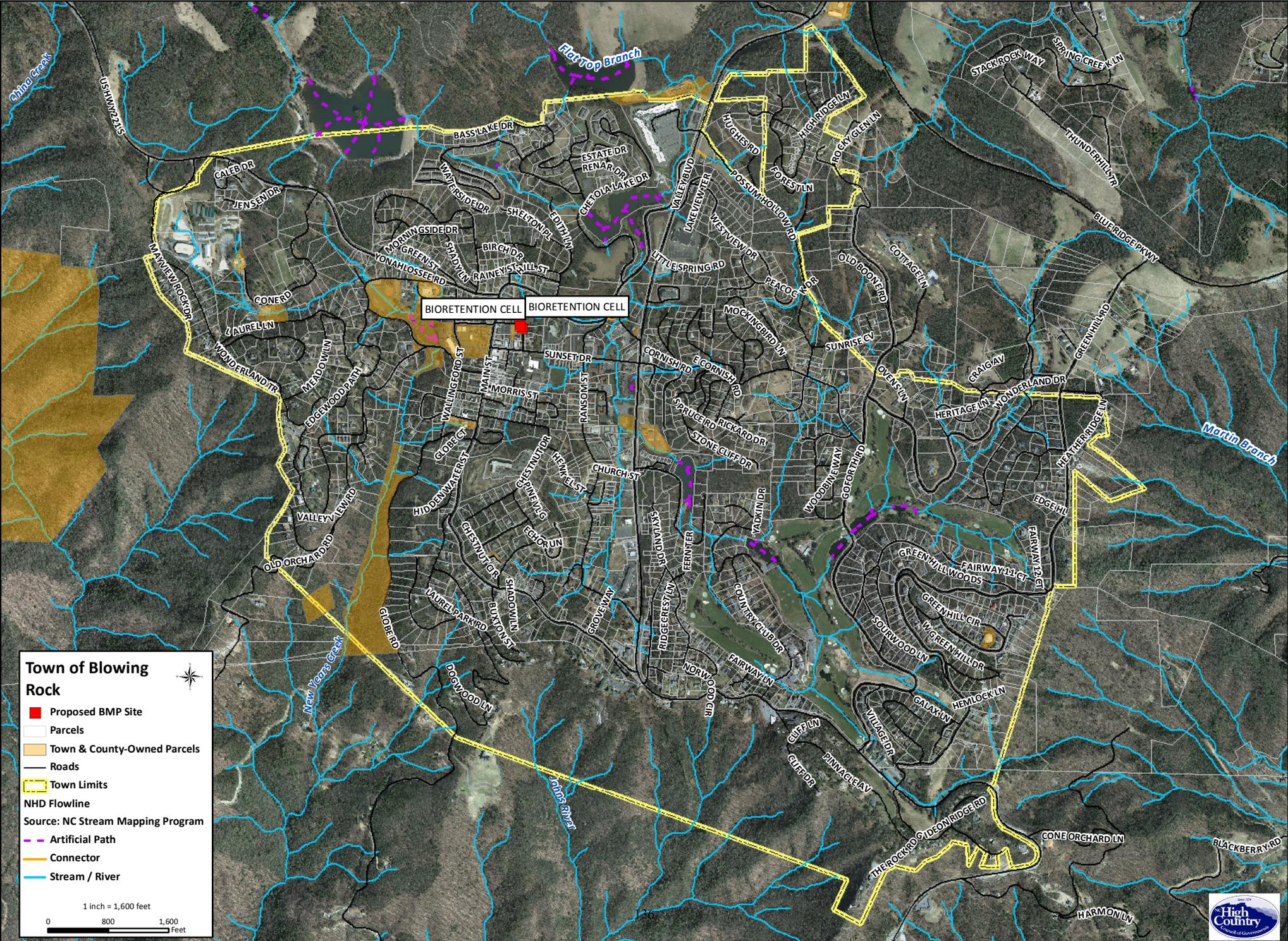
Town of Beech Mountain



- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 2,800 feet
 0 1,400 2,800
 Feet





BIORETENTION CELL BIORETENTION CELL

Flat Top Branch

Martin Branch

New Years Creek

Johns River

Chico Creek

US HWY 216

STACKROEL WAY SPRINGCREEK LN THUNDERHILL TR

CALEB DR JENSEN DR

BASS LAKE DR WYERSIDE DR

ESTATE DR RENAR DR

HUGHES DR HIGHLAND LN

ROCKY GLEN LN

MORNINGSIDE DR BIRCH DR

SHELTON PL EDITH LN

CHEYOLA LAKE DR VALLEY BLVD

LAKEVIEW TER WESTVIEW DR

POSSUM HOLLOW RD FOREST LN

OLD BOON RD COTTAGE LN

BLUE RIDGE PKWY

MAYVIEW ROCK DR CONER DR

LAUREL LN WONDERLAND DR

MEADOW LN EDENWOOD PATH

WALLINGFORD ST MAIN ST

SUNSET DR MORRIS ST

CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

SUNRISE CV OLD BOON RD

OVENS LN COTTAGE LN

GREENHILL DR

WONDERLAND DR

MEADOW LN EDENWOOD PATH

WALLINGFORD ST MAIN ST

SUNSET DR MORRIS ST

CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

SUNRISE CV OLD BOON RD

OVENS LN COTTAGE LN

GREENHILL DR

WONDERLAND DR

MEADOW LN EDENWOOD PATH

WALLINGFORD ST MAIN ST

SUNSET DR MORRIS ST

CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

SUNRISE CV OLD BOON RD

OVENS LN COTTAGE LN

GREENHILL DR

WONDERLAND DR

MEADOW LN EDENWOOD PATH

WALLINGFORD ST MAIN ST

SUNSET DR MORRIS ST

CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

SUNRISE CV OLD BOON RD

OVENS LN COTTAGE LN

GREENHILL DR

WONDERLAND DR

MEADOW LN EDENWOOD PATH

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WONDERLAND DR

MEADOW LN EDENWOOD PATH

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SUNSET DR MORRIS ST

CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

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MEADOW LN EDENWOOD PATH

WALLINGFORD ST MAIN ST

SUNSET DR MORRIS ST

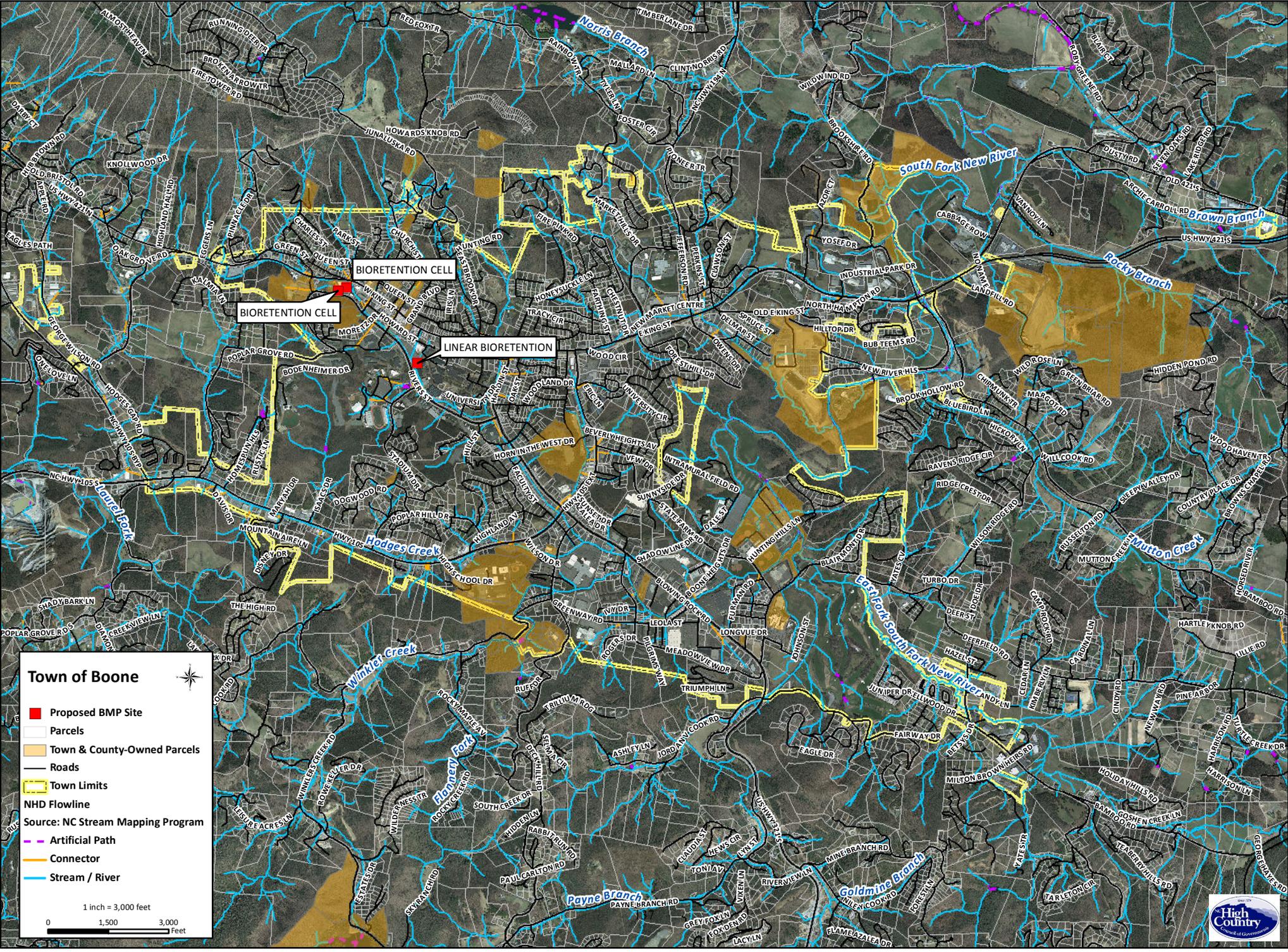
CORNISH RD E CORNISH RD

MOCKINGBIRD LN PEACOCK DR

SUNRISE CV OLD BOON RD

OVENS LN COTTAGE LN

GREENHILL DR

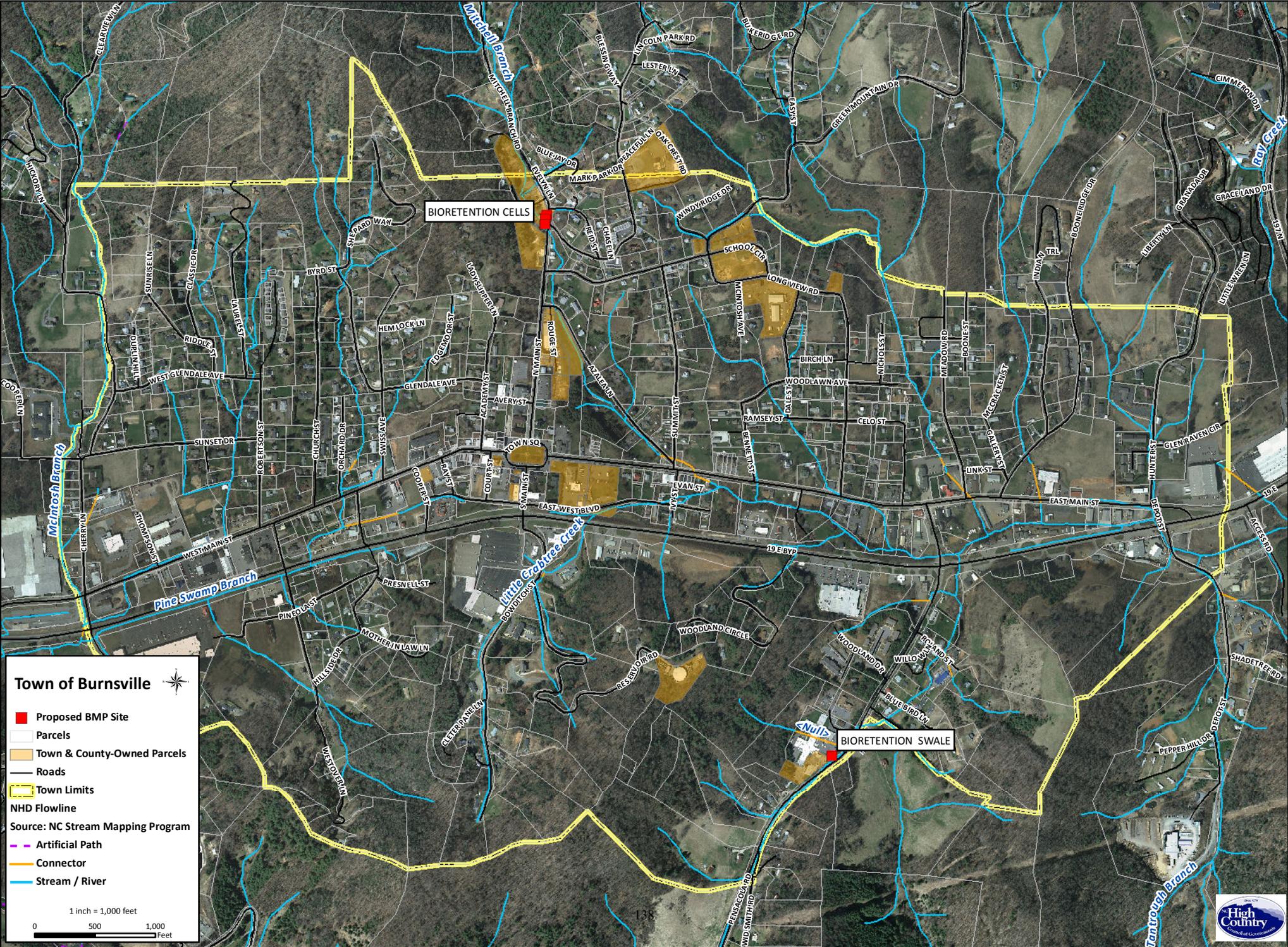


Town of Boone

- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 3,000 feet
 0 1,500 3,000 Feet





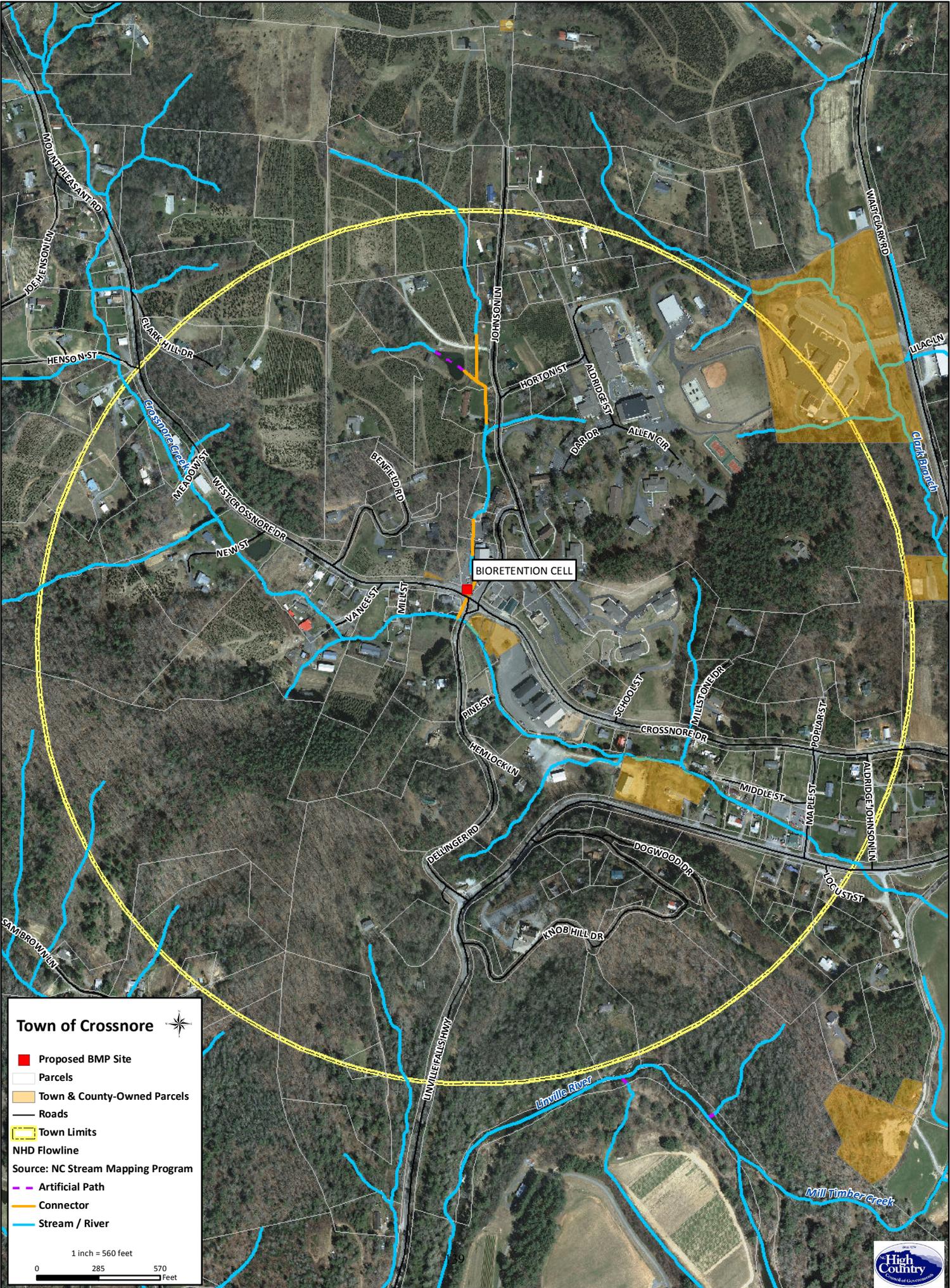
Town of Burnsville



- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

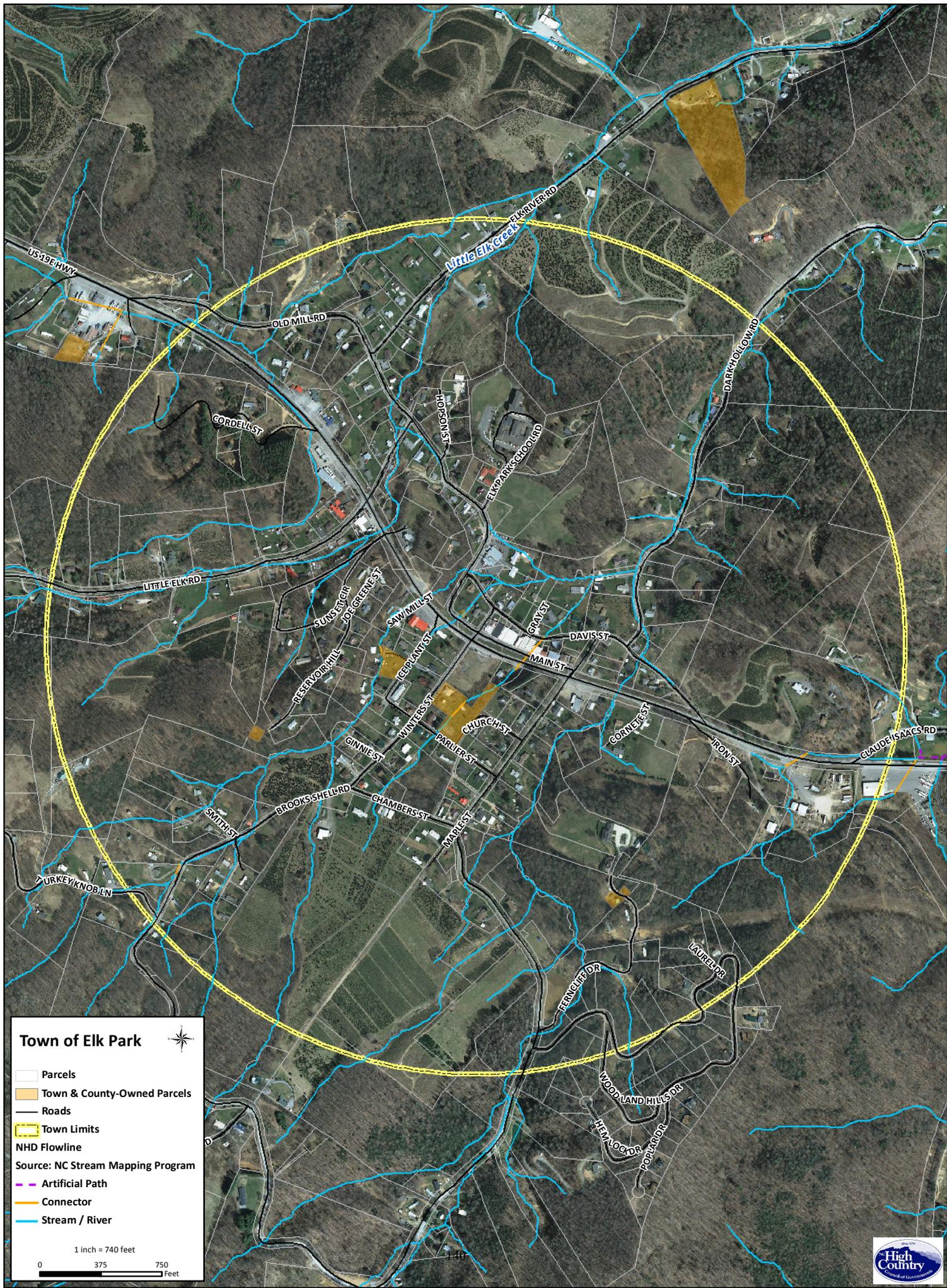
1 inch = 1,000 feet





BIORETENTION CELL

JOHNSON LN
MOUNT PLEASANT RD
JOE HENSON LN
HENSON ST
CLARK HILL DR
GROSSNORE CREEK
MEADOWS ST
WEST GROSSNORE DR
NEW ST
VANCE ST
MILL ST
DENFORD RD
JOHNSON LN
HORTON ST
ALDRIDGE ST
DAR DR
ALLEN CIR
MILLSTONE DR
SCHOOL ST
GROSSNORE DR
POPLAR ST
AUBRIDGE JOHNSON LN
MIDDLE ST
MAPLE ST
LOCUST ST
DOGWOOD DR
KNOB HILL DR
DELLINGER RD
UNIVILLE FALLS HWY
CANTERBURY LN
MILL TIMBER CREEK
CLARK BRANCH



Town of Elk Park 

-  Parcels
-  Town & County-Owned Parcels
-  Roads
-  Town Limits
-  NHD Flowline

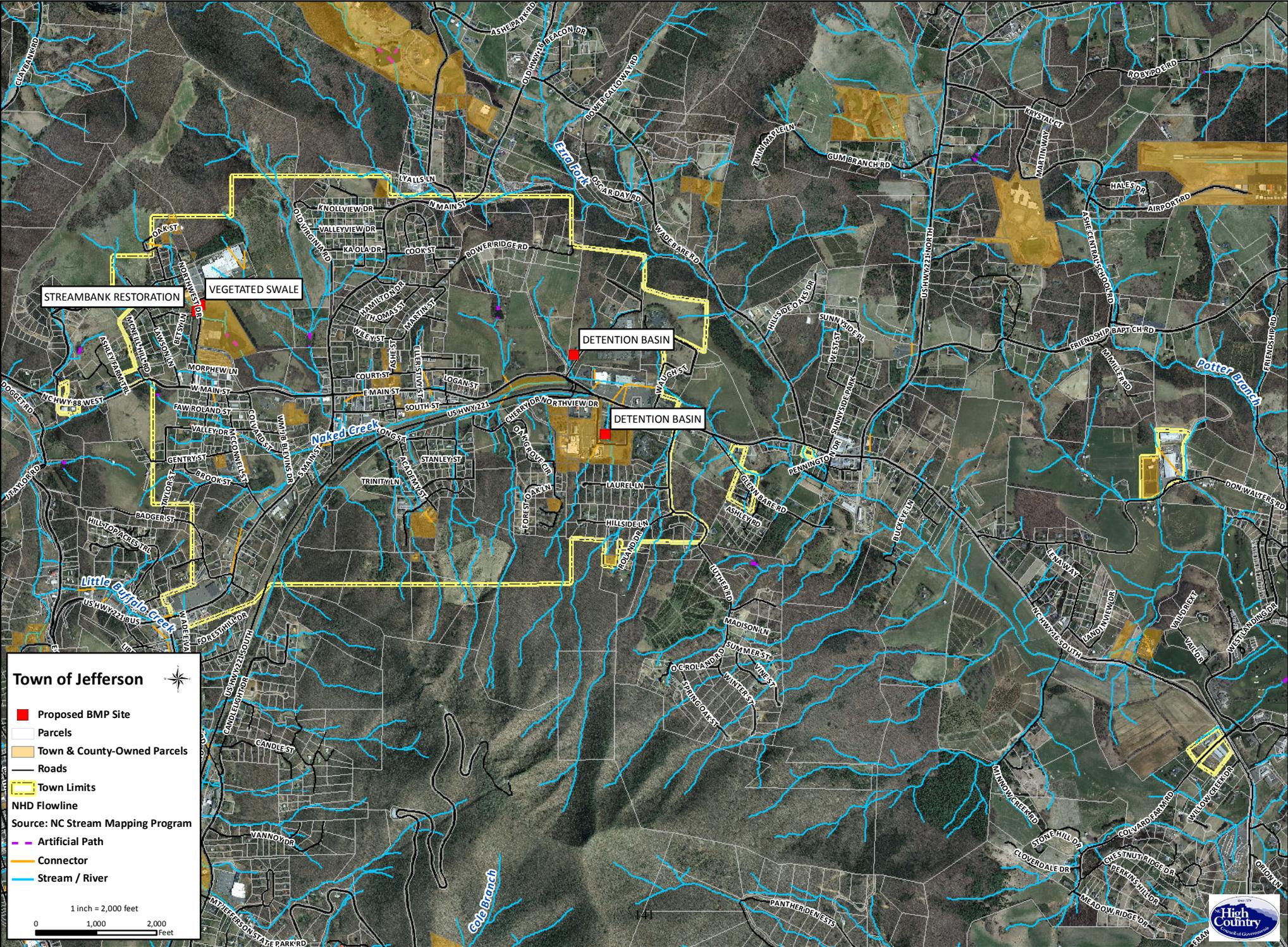
Source: NC Stream Mapping Program

-  Artificial Path
-  Connector
-  Stream / River

1 inch = 740 feet

0 375 750 Feet





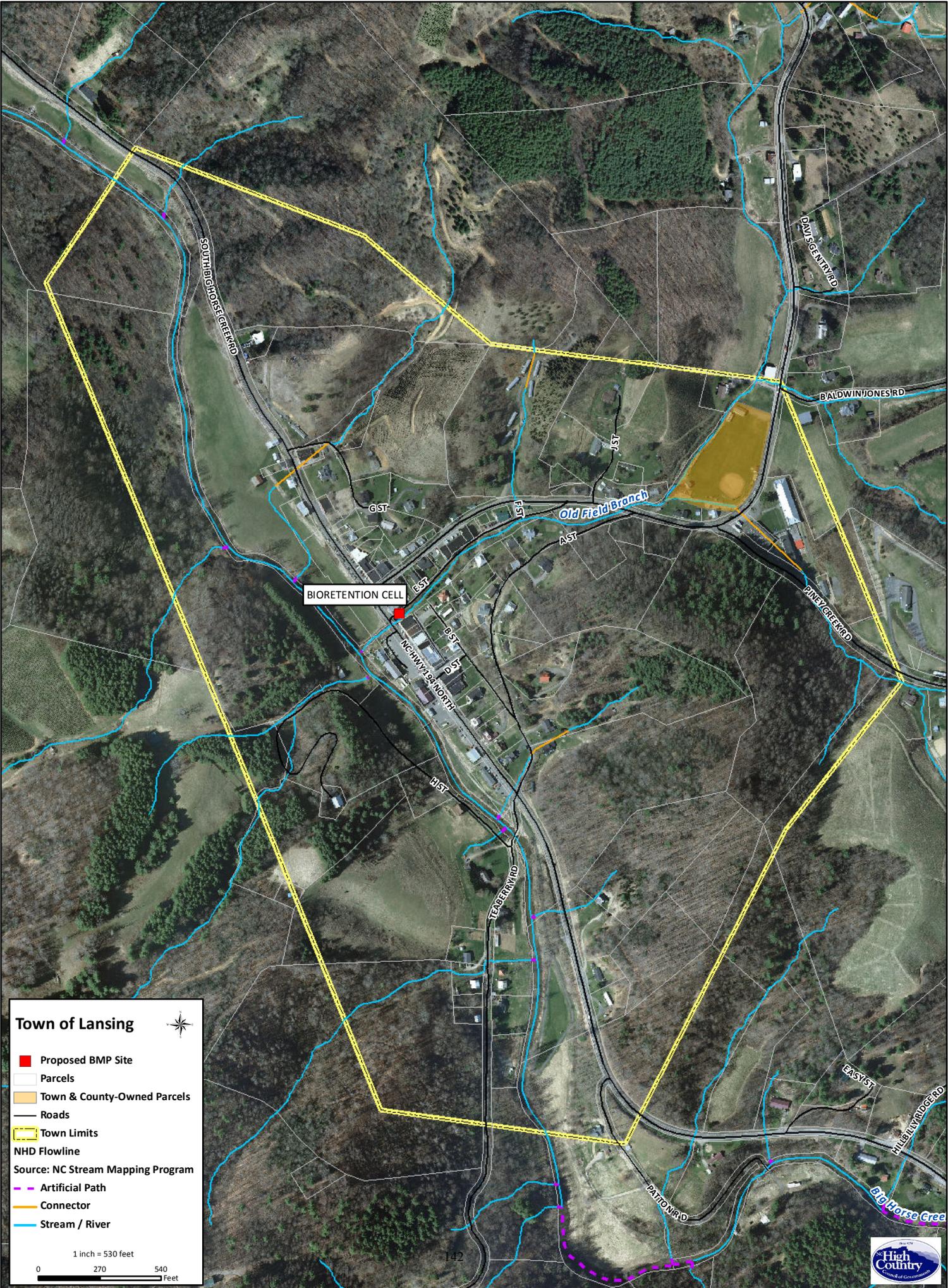
Town of Jefferson



- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 2,000 feet



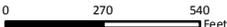


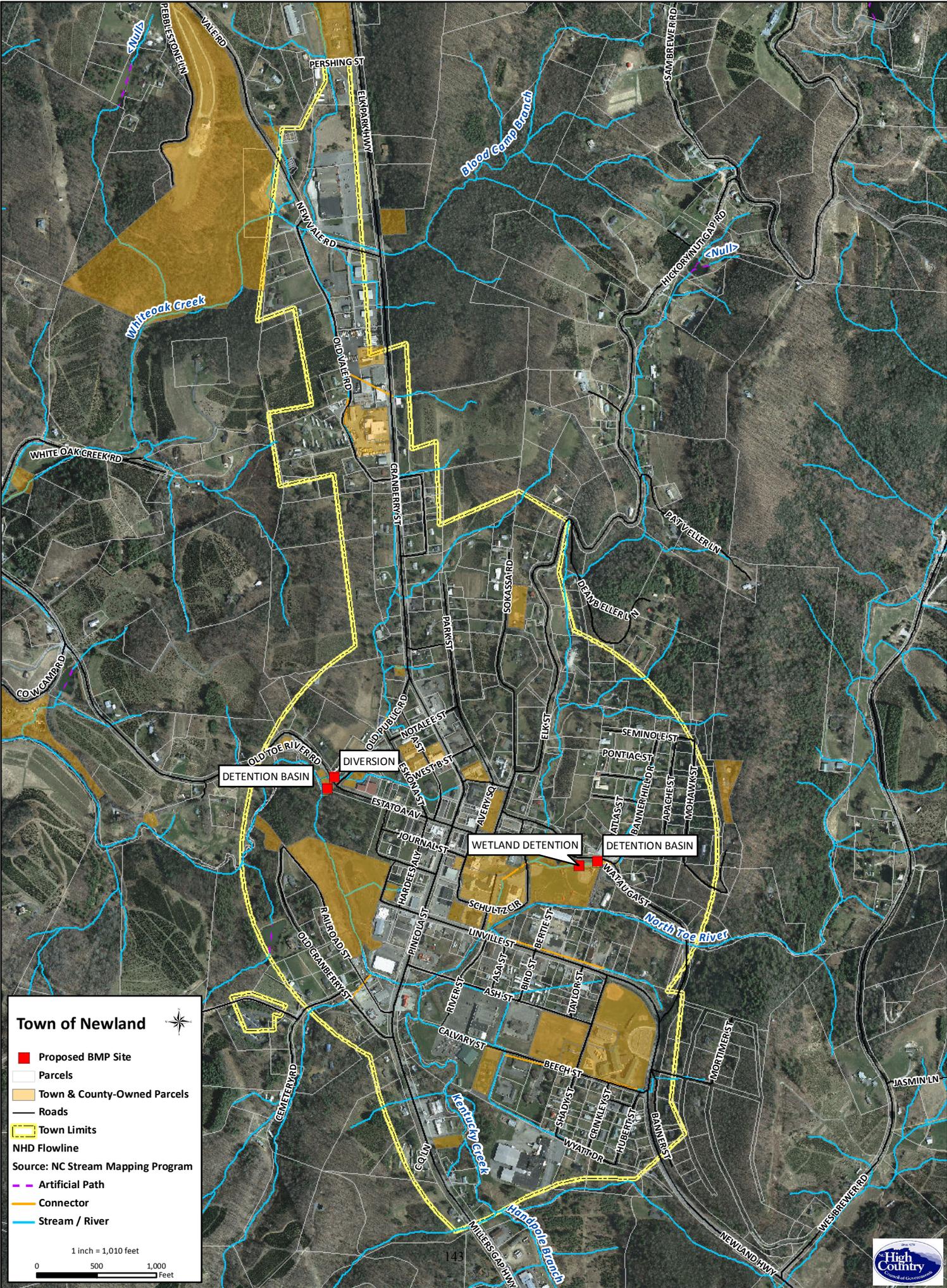
Town of Lansing



- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 530 feet





Whiteoak Creek

Blood Camp Branch

North Toe River

Kentucky Creek

Handpole Branch

DETECTION BASIN

DIVERSION

WETLAND DETENTION

DETECTION BASIN

PERSHING ST

EMKIPARK HWY

NEW VALERD

OLD VALERD

GRANBERR ST

PARR ST

OLD PUBLIC RD

ESTATO AVE

RAIFORD ST

GEMETERY RD

CO LN

NEW VALERD

OLD VALERD

GRANBERR ST

PARR ST

OLD PUBLIC RD

ESTATO AVE

RAIFORD ST

OLD GRANBERR ST

RAIFORD ST

OLD GRANBERR ST

NEW VALERD

OLD VALERD

GRANBERR ST

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NEW VALERD

OLD VALERD

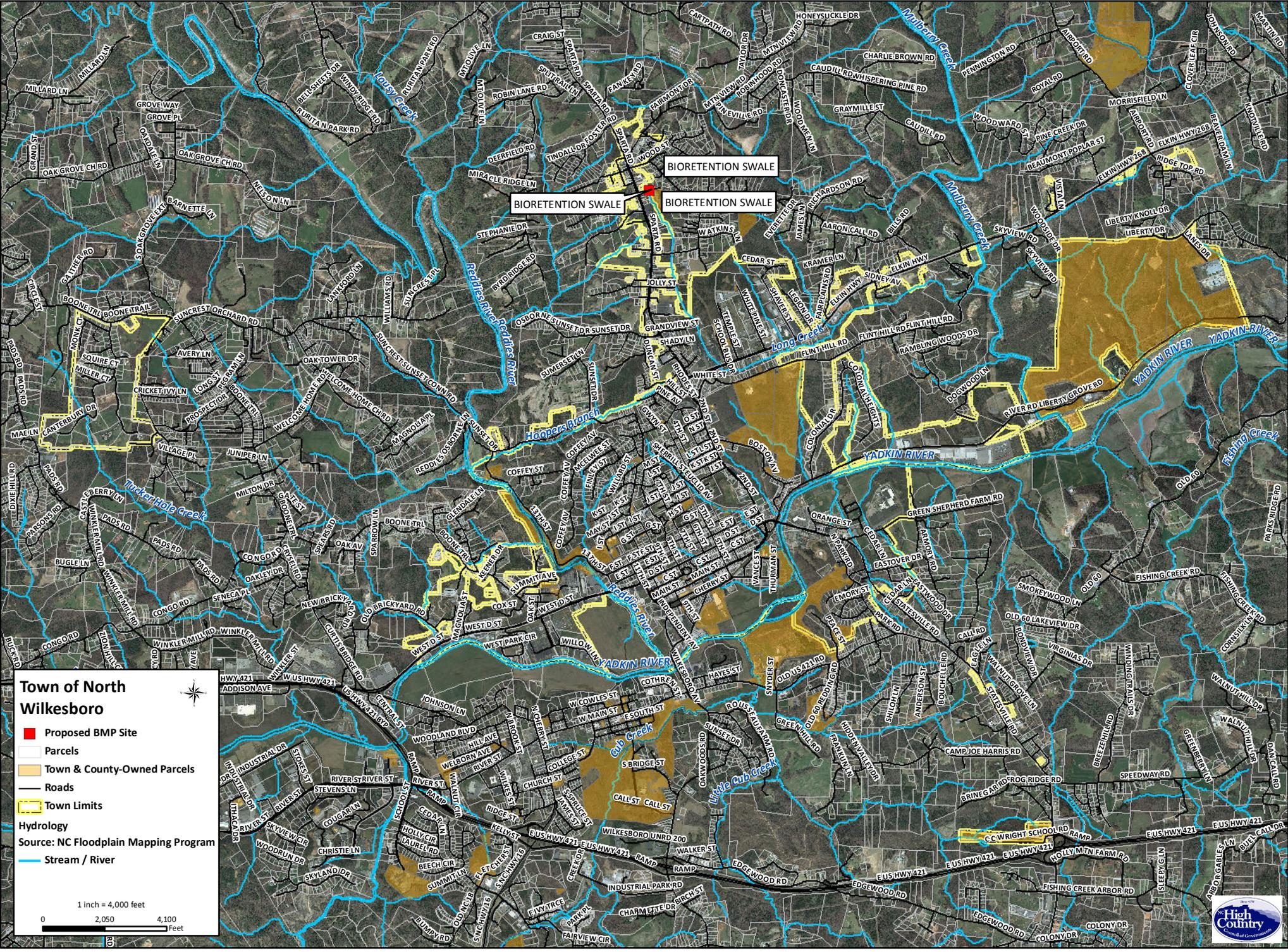
GRANBERR ST

PARR ST

OLD PUBLIC RD

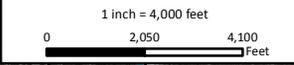
ESTATO AVE

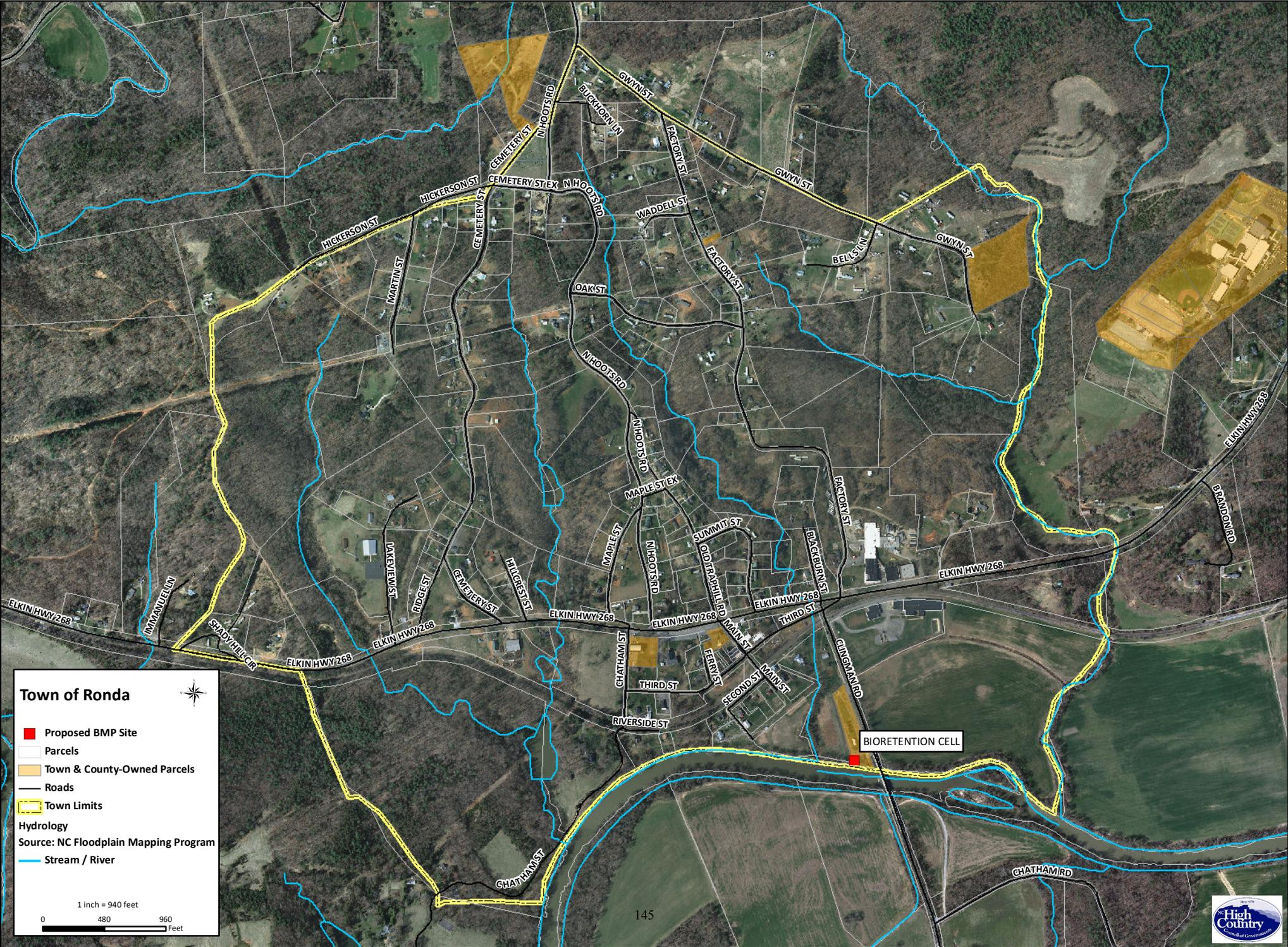
RAIFORD ST



Town of North Wilkesboro

- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- Hydrology**
- Source: NC Floodplain Mapping Program
- Stream / River

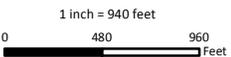




Town of Ronda



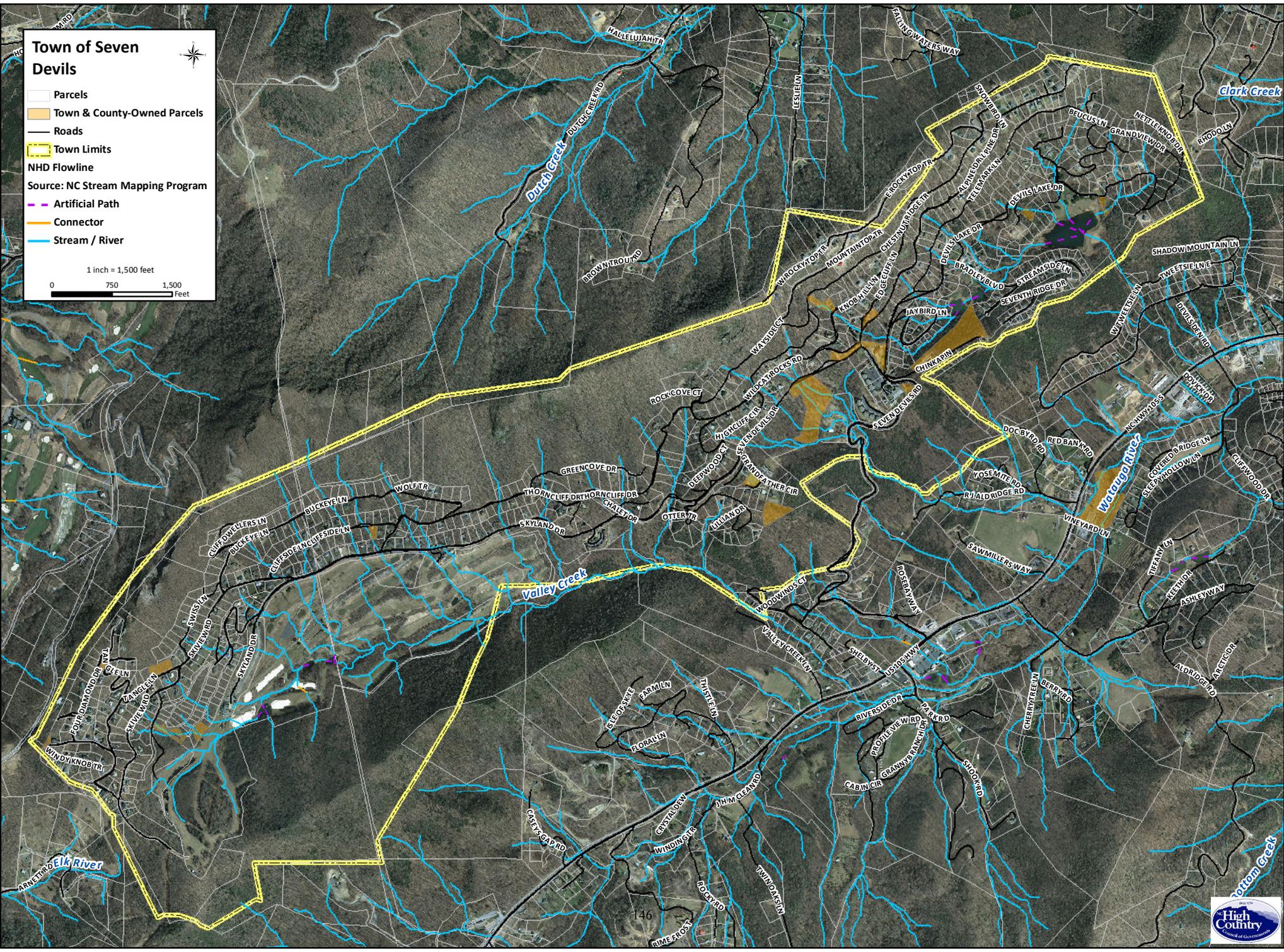
- Proposed BMP Site
- ▭ Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- Hydrology**
- Source: NC Floodplain Mapping Program
- Stream / River

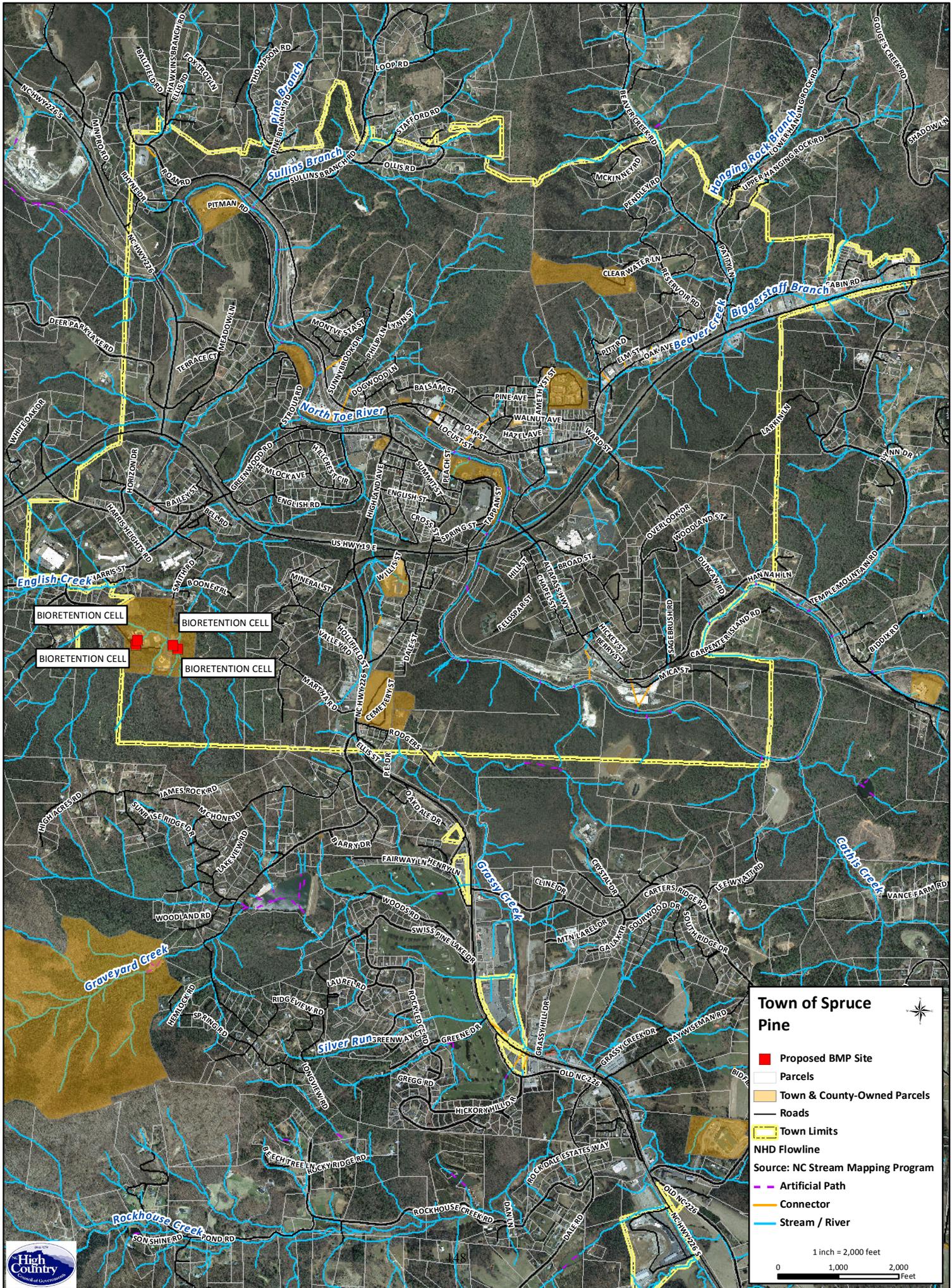


Town of Seven Devils

- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 1,500 feet
0 750 1,500 Feet





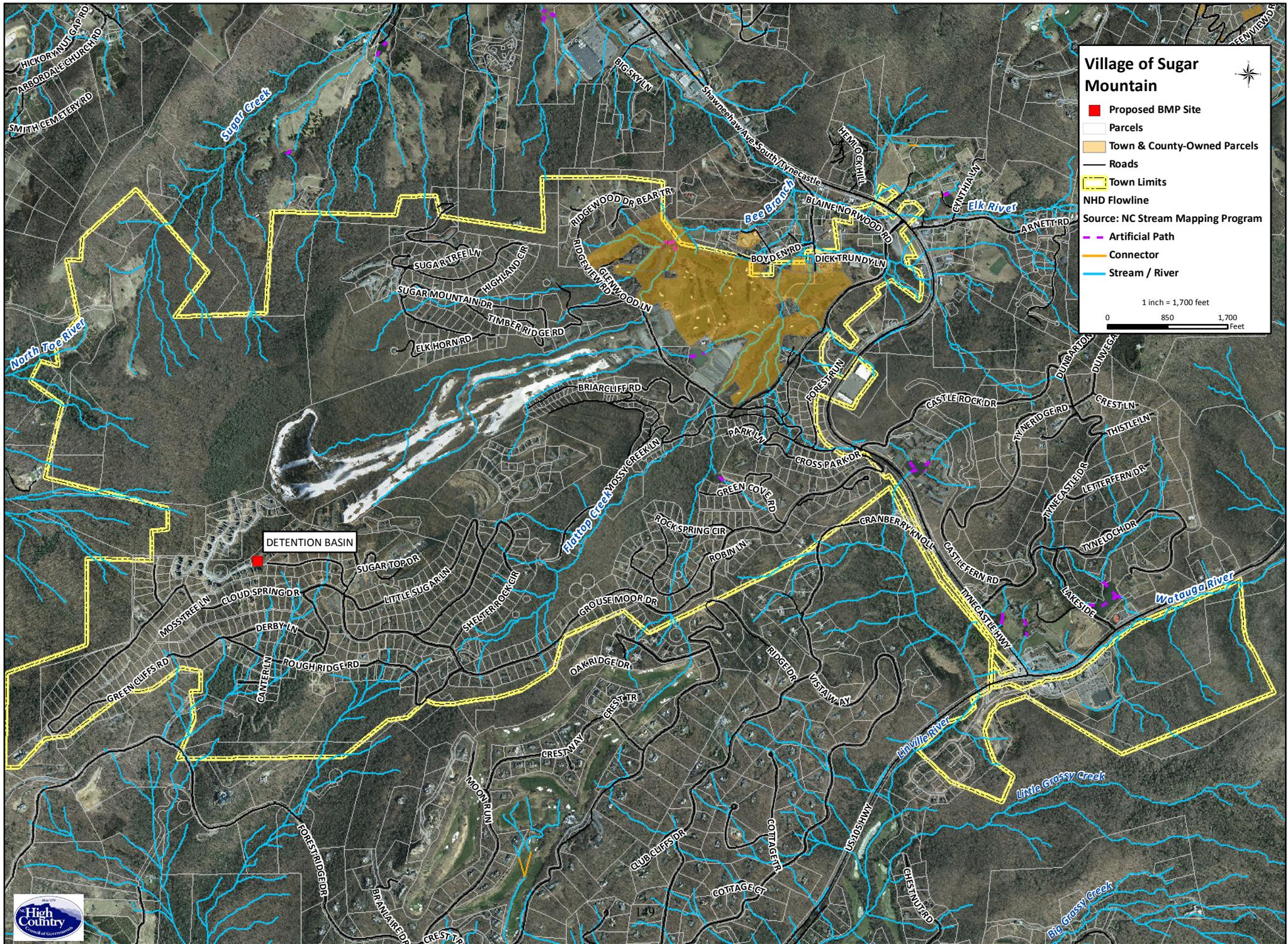
Town of Spruce Pine

- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline
- Source: NC Stream Mapping Program
- Artificial Path
- Connector
- Stream / River

1 inch = 2,000 feet

0 1,000 2,000 Feet





Village of Sugar Mountain

- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits
- NHD Flowline

Source: NC Stream Mapping Program

- Artificial Path
- Connector
- Stream / River

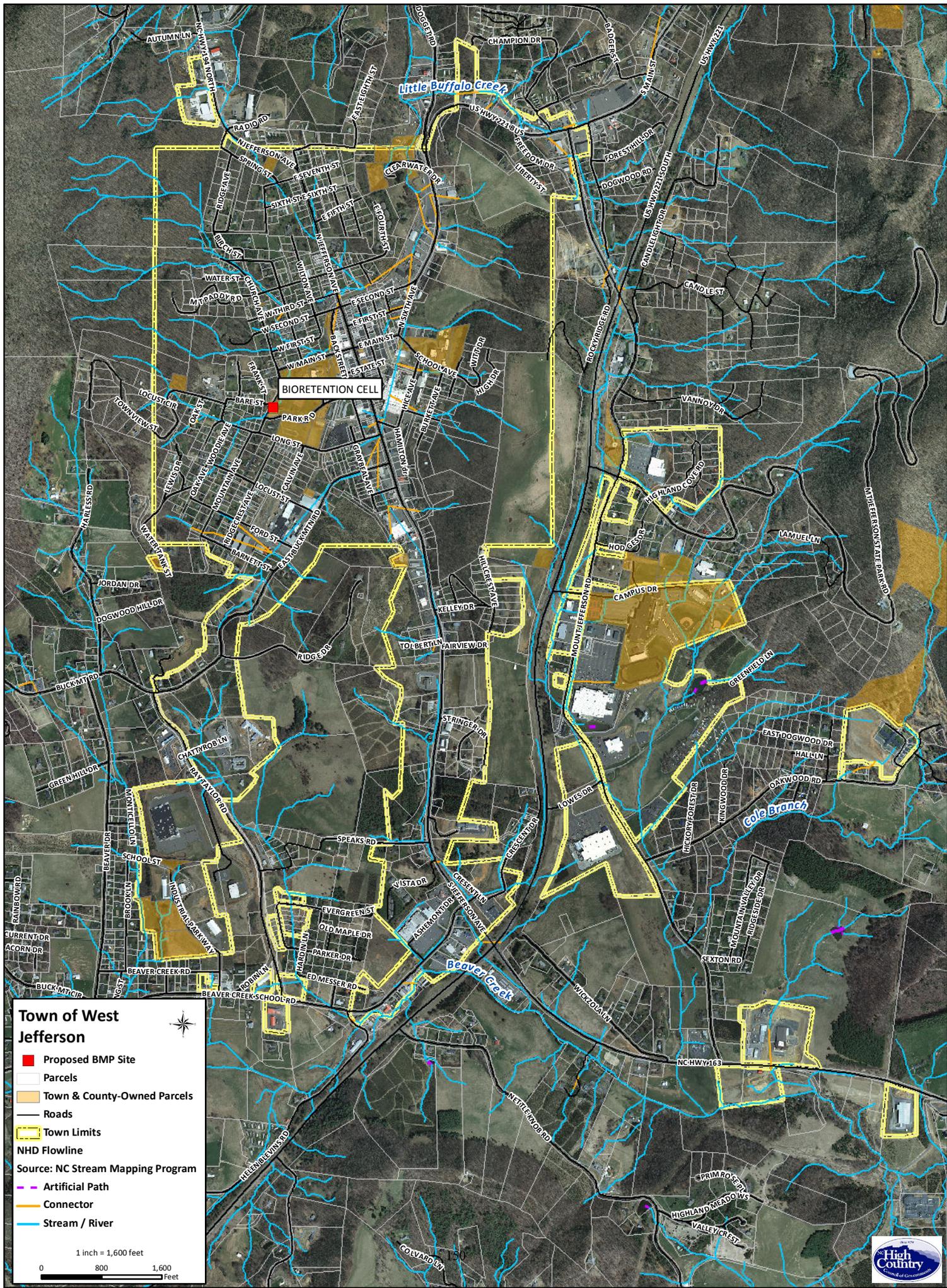
1 inch = 1,700 feet

0 850 1,700
Feet



DETENTION BASIN

149



BIORETENTION CELL

Little Buffalo Creek

Cole Branch

Beaver Creek





Town of Wilkesboro

- Proposed BMP Site
- Parcels
- Town & County-Owned Parcels
- Roads
- Town Limits

Hydrology

Source: NC Floodplain Mapping Program

— Stream / River

1 inch = 3,700 feet

0 1,850 3,700 Feet

